

2016 CALIFORNIA GREEN BUILDING STANDARDS CODE NONRESIDENTIAL MANDATORY MEASURES

SIGNOFF

INSPECTOR SIGNOFF **CHAPTER 3** SIGNOFF **GREEN BUILDING SECTION 301 GENERAL 301.1 SCOPE.** Buildings shall be designed to include the green building measures specified as mandatory in the application checklists contained in this code. Voluntary green building measures are also included in the application checklists and may be included in the design and construction of structures covered by this code, but are not required unless adopted by a city, county, or city and county as specified in Section 101.7. **ABBREVIATION DEFINITIONS:** Department of Housing and Community Development California Building Standards Commission **5.504.4 Finish material pollutant control.** Finish materials shall comply with Sections 5.504.4.1 through 5.504.4.4. Division of the State Architect, Structural Safety Office of Statewide Health Planning and Development Low Rise High Rise Additions and Alterations (chloroform, ethylene dichloride, methylene chloride, perchloroethylene and trichloroethylene), except for aerosol products as specified in subsection 2, below. NONRESIDENTIAL MANDATORY MEASURES **DIVISION 5.1 PLANNING AND DESIGN SECTION 5.101 GENERAL** The provisions of this chapter outline planning, design and development methods that include environmentally responsible site selection, building design, building siting and development to protect, restore and enhance the environmental quality of the site and respect the integrity of adjacent properties. **SECTION 5.102 DEFINITIONS** The following terms are defined in Chapter 2 (and are included here for reference) **TENANT-OCCUPANTS.** Building occupants who inhabit a building during its normal hours of operation as permanent occupants, such as employees, as distinguished from customers and other transient visitors. DIVISION 5.4 MATERIAL CONSERVATION AND RESOURCE EFFICIENCY SECTION 5.408 CONSTRUCTION WASTE REDUCTION, DISPOSAL AND **5.408.1 CONSTRUCTION WASTE MANAGEMENT.** Recycle and/or salvage for reuse a minimum of 50% of the non-hazardous construction and demolition waste in accordance with Section 5.408.1.1, 5.408.1.2 or 5.408.1.3; or meet a local construction and demolition waste management ordinance, whichever is more stringent. **5.408.1.1 Construction waste management plan.** Where a local jurisdiction does not have a construction and demolition waste management ordinance, submit a construction waste management plan that: 1. Identifies the construction and demolition waste materials to be diverted from disposal by efficient usage, recycling, reuse on the project or salvage for future use or sale. 2. Determines if construction and demolition waste materials will be sorted on-site (source-separated) or 3. Identifies diversion facilities where construction and demolition waste material collected will be taken. 4. Specifies that the amount of construction and demolition waste materials diverted shall be calculated by weight or volume, but not by both. **5.408.1.2 Waste Management Company.** Utilize a waste management company that can provide verifiable documentation that the percentage of construction and demolition waste material diverted from the landfill Note: The owner or contractor shall make the determination if the construction and demolition waste material will be diverted by a waste management company. **Exceptions to Sections 5.408.1.1 and 5.408.1.2:** Excavated soil and land-clearing debris. . Alternate waste reduction methods developed by working with local agencies if diversion or recycle facilities capable of compliance with this item do not exist. 3. Demolition waste meeting local ordinance or calculated in consideration of loacl recycleing facilities **5.408.1.3 Waste stream reduction alternative.** The combined weight of new construction disposal that does not exceed two pounds per square foot of building area may be deemed to meet the 50% minimum requirement as approved by the enforcing agency. **5.408.1.4 Documentation.** Documentation shall be provided to the enforcing agency which demonstrates compliance with Sections 5.408.1.1, through 5.408.1.3. The waste management plan shall be updated as necessary and shall be accessible during construction for examination by the enforcing agency. 1. Sample forms found in "A Guide to the California Green Building Standards Code (Nonresidential)" located at www.bsc.ca.gov/Home/CALGreen.aspx may be used to assist in documenting compliance with the 2. Mixed construction and demolition debris processors can be located at the California Department of Resources Recycling and Recovery (CalRecycle).

SECTION 5.504 POLLUTANT CONTROL 5.504.1.3 Temporary ventilation. The permanent HVAC system shall only be used during construction if necessary to condition the building or areas of addition or alteration within the required temperature range for material and equipment installation. If the HVAC system is used during construction, use return air filters with a Minimum Efficiency Reporting Value (MERV) of 8, based on ASHRAE 52.2-1999, or an average efficiency of 30% based on ASHRAE 52.1-1992 Replace all filters immediately prior to occupancy, or, if the building is occupied during alteration, at the conclusion of construction.

5.504.3 Covering of duct openings and protection of mechanical equipment during construction. At the time of rough installation, or during storage on the construction site and until final startup of the heating, cooling and ventilating equipment, all duct and other related air distribution component openings shall be covered with tape, plastic, sheet metal or other methods acceptable to the enforcing agency to reduce the amount of dust, water and debris which may collect in the system.

5.504.4.1 Adhesives, sealants and caulks. Adhesives, sealants, and caulks used on the project shall meet the requirements of the following standards: 1. Adhesives, adhesive bonding primers adhesive primers, sealants, sealant primers and caulks shall comply with local or regional air pollution control or air quality management district rules where applicable, or SCAQMD Rule 1168 VOC limits, as shown in Tables 5.504.4.1 and 5.504.4.2. Such products also shall comply with the Rule 1168 prohibition on the use of certain toxic compounds

2. Aerosol adhesives, and smaller unit sizes of adhesives, and sealant or caulking compounds (in units of product, less packaging, which do not weigh more than one pound and do not consist of more than 16 fluid ounces) shall comply with statewide VOC standards and other requirements, including prohibitions on use of certain toxic compounds, of *California Code of Regulations*, Title 17, commencing with Section 94507

Less Water and Less Exempt Compounds in Grams p	per Liter
ARCHITECTURAL APPLICATIONS	CURRENT VOC LIMIT
INDOOR CARPET ADHESIVES	50
CARPET PAD ADHESIVES	50
OUTDOOR CARPET ADHESIVES	150
WOOD FLOORING ADHESIVES	100
RUBBER FLOOR ADHESIVES	60
SUBFLOOR ADHESIVES	50
CERAMIC TILE ADHESIVES	65
VCT & ASPHALT TILE ADHESIVES	50
DRYWALL & PANEL ADHESIVES	50
COVE BASE ADHESIVES	50
MULTIPURPOSE CONSTRUCTION ADHESIVES	70
STRUCTURAL GLAZING ADHESIVES	100
SINGLE-PLY ROOF MEMBRANE ADHESIVES	250
OTHER ADHESIVES NOT SPECIFICALLY LISTED	50
SPECIALTY APPLICATIONS	
PVC WELDING	510
CPVC WELDING	490
ABS WELDING	325
PLASTIC CEMENT WELDING	250
ADHESIVE PRIMER FOR PLASTIC	550
CONTACT ADHESIVE	80
SPECIAL PURPOSE CONTACT ADHESIVE	250
STRUCTURAL WOOD MEMBER ADHESIVE	140
TOP & TRIM ADHESIVE	250
SUBSTRATE SPECIFIC APPLICATIONS	
METAL TO METAL	30
PLASTIC FOAMS	50
POROUS MATERIAL (EXCEPT WOOD)	50

1. IF AN ADHESIVE IS USED TO BOND DISSIMILAR SUBSTRATES TOGETHER, THE ADHESIVE WITH THE HIGHEST VOC CONTENT SHALL BE ALLOWED. 2. FOR ADDITIONAL INFORMATION REGARDING METHODS TO MEASURE THE VOC CONTENT SPECIFIED IN THIS TABLE, SEE SOUTH COAST AIR QUALITY MANAGEMENT DISTRICT RULE 1168,

www.arb.ca.gov/DRDB/SC/CURHTML/R1168.PDF

ΓABLE 5.504.4.2 - SEALANT VOC LI	MIT
ess Water and Less Exempt Compounds in Gran	ns per Liter
SEALANTS	CURRENT VOC LIMIT
ARCHITECTURAL	250
MARINE DECK	760
NONMEMBRANE ROOF	300
ROADWAY	250
SINGLE-PLY ROOF MEMBRANE	450
OTHER	420
SEALANT PRIMERS	
ARCHITECTURAL	
NONPOROUS	250
POROUS	775
MODIFIED BITUMINOUS	500
MARINE DECK	760
OTHER	750

NOTE: FOR ADDITIONAL INFORMATION REGARDING METHODS TO MEASURE THE VOC CONTENT SPECIFIED IN THESE TABLES, SEE SOUTH COAST AIR QUALITY MANAGEMENT DISTRICT RULE 1168.

5.504.4.3 Paints and coatings. Architectural paints and coatings shall comply with VOC limits in Table 1 of the ARB Architectural Coatings Suggested Control Measure, as shown in Table 5.504.4.3, unless more stringent local limits apply. The VOC content limit for coatings that do not meet the definitions for the specialty coatings categories listed in Table 5.504.4.3 shall be determined by classifying the coating as a Flat, Nonflat or Nonflat-High Gloss coating, based on its gloss, as defined in Subsections 4.21, 4.36 and 4.37 of the 2007 California Air Resources Board Suggested Control Measure, and the corresponding Flat, Nonflat or Nonflat-High Gloss VOC limit in Table 5.504.4.3 shall apply.

5.504.4.3.1 Aerosol Paints and coatings. Aerosol paints and coatings shall meet the PWMIR Limits for ROC in Section 94522(a)(3) and other requirements, including prohibitions on use of certain toxic compounds and ozone depleting substances, in Sections 94522(c)(2) and (d)(2) of California Code of Regulations, Title 17, commencing with Section 94520; and in areas under the jurisdiction of the Bay Area Air Quality Management District additionally comply with the percent VOC by weight of product limits of Regulation 8

GRAMS OF VOC PER LITER OF COATING, LESS WATER & LESS EXEMPT COMPOUNDS COATING CATEGORY **CURRENT VOC LIMIT** FLAT COATINGS 50 NONFLAT COATINGS 100 NONFLAT HIGH GLOSS COATINGS 150 SPECIALTY COATINGS ALUMINUM ROOF COATINGS 400 BASEMENT SPECIALTY COATINGS 400 BITUMINOUS ROOF COATINGS 50 BITUMINOUS ROOF PRIMERS 350 BOND BREAKERS 350 CONCRETE CURING COMPOUNDS 350 CONCRETE/MASONRY SEALERS 100 DRIVEWAY SEALERS 50 DRY FOG COATINGS 150 FAUX FINISHING COATINGS 350 FIRE RESISTIVE COATINGS 350 FLOOR COATINGS 100 FORM-RELEASE COMPOUNDS 250 GRAPHIC ARTS COATINGS (SIGN PAINTS) 500 HIGH-TEMPERATURE COATINGS 420 INDUSTRIAL MAINTENANCE COATINGS 250 LOW SOLIDS COATINGS₁ 120 MAGNESITE CEMENT COATINGS 450 MASTIC TEXTURE COATINGS 100 METALLIC PIGMENTED COATINGS 500 MULTICOLOR COATINGS 250 PRETREATMENT WASH PRIMERS 420 PRIMERS, SEALERS, & UNDERCOATERS 100 REACTIVE PENETRATING SEALERS 350 RECYCLED COATINGS 250 ROOF COATINGS 50 RUST PREVENTATIVE COATINGS 250 SHELLACS: CLEAR 730 550 SPECIALTY PRIMERS, SEALERS & UNDERCOATERS 100 250 STONE CONSOLIDANTS 450 SWIMMING POOL COATINGS TRAFFIC MARKING COATINGS 100 TUB & TILE REFINISH COATINGS 420 WATERPROOFING MEMBRANES 250 WOOD COATINGS 275 WOOD PRESERVATIVES 350 ZINC-RICH PRIMERS 1. GRAMS OF VOC PER LITER OF COATING, INCLUDING WATER & EXEMPT COMPOUNDS

TABLE 5.504.4.3 - VOC CONTENT LIMITS FOR ARCHITECTURAL

2. THE SPECIFIED LIMITS REMAIN IN EFFECT UNLESS REVISED LIMITS ARE LISTED IN SUBSEQUENT COLUMNS IN 3. VALUES IN THIS TABLE ARE DERIVED FROM THOSE SPECIFIED BY THE CALIFORNIA AIR RESOURCES BOARD, ARCHITECTURAL COATINGS SUGGESTED CONTROL MEASURE, FEB. 1, 2008. MORE INFORMATION IS AVAILABLE

FROM THE AIR RESOURCES BOARD. **5.504.4.3.2 Verification.** Verification of compliance with this section shall be provided at the request of the enforcing agency. Documentation may include, but is not limited to, the following: Manufacturer's product specification

Field verification of on-site product containers **5.504.4.5 Composite wood products.** Hardwood plywood, particleboard and medium density fiberboard composite wood products used on the interior or exterior of the buildings shall meet the requirements for formaldehyde as specified in ARB's Air Toxics Control Measure for Composite Wood (17 CCR 93120 et seq.). Those materials not exempted under the ATCM must meet the specified emission limits, as shown in Table

5.504.4.5.3 Documentation. Verification of compliance with this section shall be provided as requested by the enforcing agency. Documentation shall include at least one of the following:

 Product certifications and specifications. Chain of custody certifications.

3. Product labeled and invoiced as meeting the Composite Wood Products regulation (see

CCR, Title 17, Section 93120, et seq.).

4. Exterior grade products marked as meeting the PS-1 or PS-2 standards of the Engineered Wood Association, the Australian AS/NZS 2269 or European 636 3S standards. 5. Other methods acceptable to the enforcing agency.

TABLE 5.504.4.5 - FORMALDEHYDE LIMITS ₁	
MAXIMUM FORMALDEHYDE EMISSIONS IN PARTS PER MI	ILLION
PRODUCT	CURRENT LIMIT
HARDWOOD PLYWOOD VENEER CORE	0.05
HARDWOOD PLYWOOD COMPOSITE CORE	0.05
PARTICLE BOARD	0.09
MEDIUM DENSITY FIBERBOARD	0.11
THIN MEDIUM DENSITY FIBERBOARD2	0.13

1. VALUES IN THIS TABLE ARE DERIVED FROM THOSE SPECIFIED BY THE CALIFORNIA AIR RESOURCES BOARD, AIR TOXICS CONTROL MEASURE FOR COMPOSITE WOOD AS TESTED IN ACCORDANCE WITH ASTM E 1333. FOR ADDITIONAL INFORMATION, SEE CALIFORNIA CODE OF REGULATIONS, TITLE 17, SECTIONS 93120 THROUGH

2. THIN MEDIUM DENSITY FIBERBOARD HAS A MAXIMUM THICKNESS OF 5/16 INCHES (8 MM).

CHAPTER 7 INSTALLER & SPECIAL INSPECTOR QUALIFICATIONS

702 QUALIFICATIONS 702.1 INSTALLER TRAINING. HVAC system installers shall be trained and certified in the proper installation of HVAC systems including ducts and equipment by a nationally or regionally recognized training or certification program. Uncertified persons may perform HVAC installations when under the direct supervision and responsibility of a person trained and certified to install HVAC systems or contractor licensed to install HVAC systems. Examples of acceptable HVAC training and certification programs include but are not limited to the following:

Public utility training programs. 3. Training programs sponsored by trade, labor or statewide energy consulting or verification

4. Programs sponsored by manufacturing organizations. 5. Other programs acceptable to the enforcing agency.

evaluating the qualifications of a special inspector:

. State certified apprenticeship programs.

NSPECTOR SIGNOFF

702.2 SPECIAL INSPECTION [HCD]. When required by the enforcing agency, the owner or the responsible entity acting as the owner's agent shall employ one or more special inspectors to provide inspection or other duties necessary to substantiate compliance with this code. Special inspectors shall demonstrate competence to the satisfaction of the enforcing agency for the particular type of inspection or task to be performed. In addition to other certifications or qualifications acceptable to the enforcing agency, the following certifications or education may be considered by the enforcing agency when

1. Certification by a national or regional green building program or standard publisher. 2. Certification by a statewide energy consulting or verification organization, such as HERS raters, building performance contractors, and home energy auditors. 3. Successful completion of a third party apprentice training program in the appropriate trade. 4. Other programs acceptable to the enforcing agency.

1. Special inspectors shall be independent entities with no financial interest in the materials or the project they are inspecting for compliance with this code. 2. HERS raters are special inspectors certified by the California Energy Commission (CEC) to rate homes in California according to the Home Energy Rating System (HERS)

[BSC] When required by the enforcing agency, the owner or the responsible entity acting as the owner's agent shall employ one or more special inspectors to provide inspection or other duties necessary to substantiate compliance with this code. Special inspectors shall demonstrate competence to the satisfaction of the enforcing agency for the particular type of inspection or task to be performed. In addition, the special inspector shall have a certification from a recognized state, national or international association, as determined by the local agency. The area of certification shall be closely related to the primary job function, as determined by the local agency.

Note: Special inspectors shall be independent entities with no financial interest in the materials or the project they are inspecting for compliance with this code.

703 VERIFICATIONS

703.1 DOCUMENTATION. Documentation used to show compliance with this code shall include but is not limited to, construction documents, plans, specifications, builder or installer certification, inspection reports, or other methods acceptable to the enforcing agency which demonstrate substantial conformance. When specific documentation or special inspection is necessary to verify compliance, that method of compliance will be specified in the appropriate section or identified applicable checklist.

Architects, Inc.

CAMPUS BUILDING PERMIT NUMBER B19-163

DARRYL K. CARTOZIAN A.I.A

DKC ARCHITECTS, INC. 31555 AVENUE E YUCAIPA, CALIFORNIA 92399 Ph. (909) 798-7900 website: dkcarch.com



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WEST LOTHIAN ROOF

PROJECT # 956399 - CANN #

UCR CAPITAL PROGRAMS 1223 UNIVERSITY AVE. RIVERSIDE, CA 92507

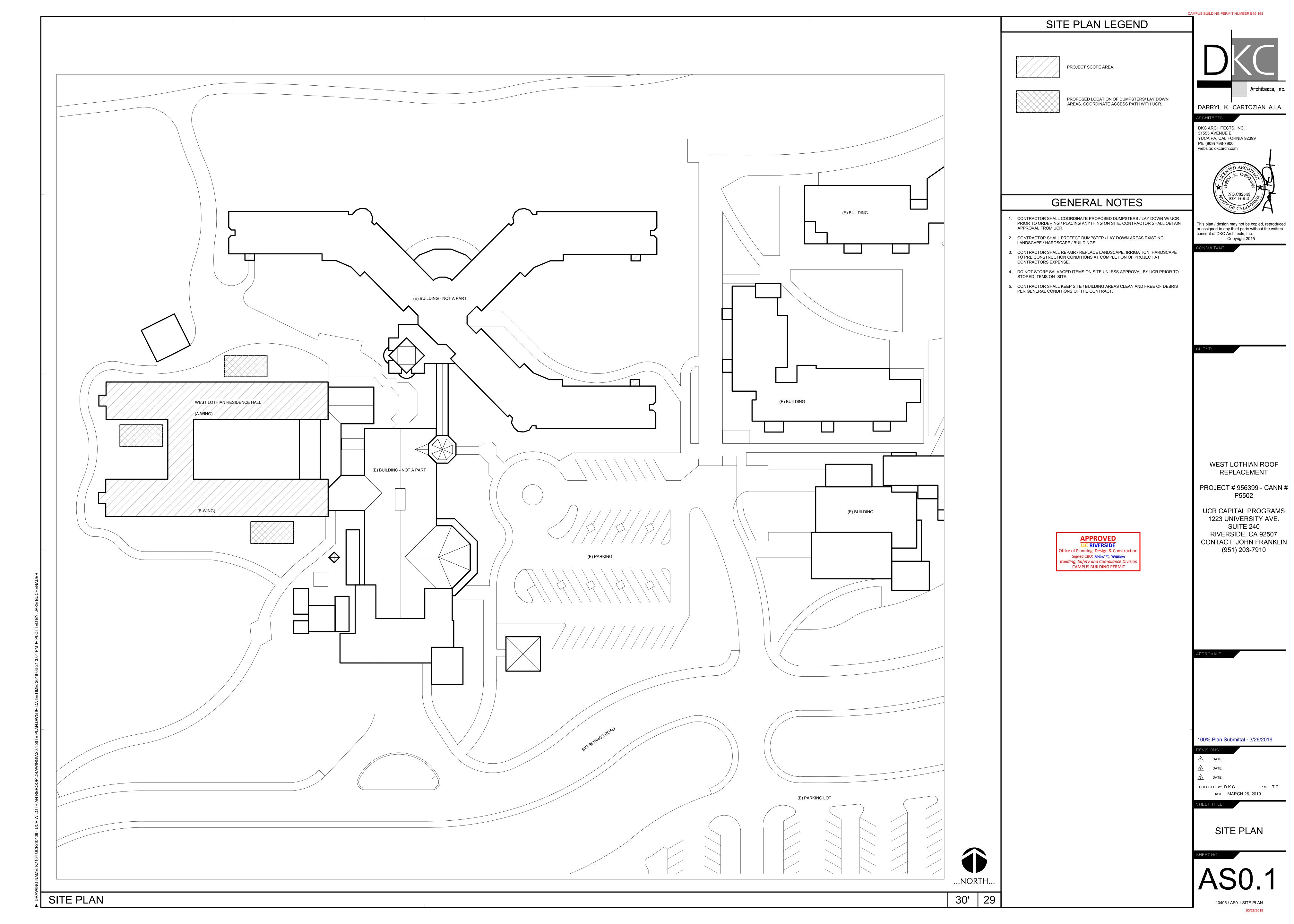
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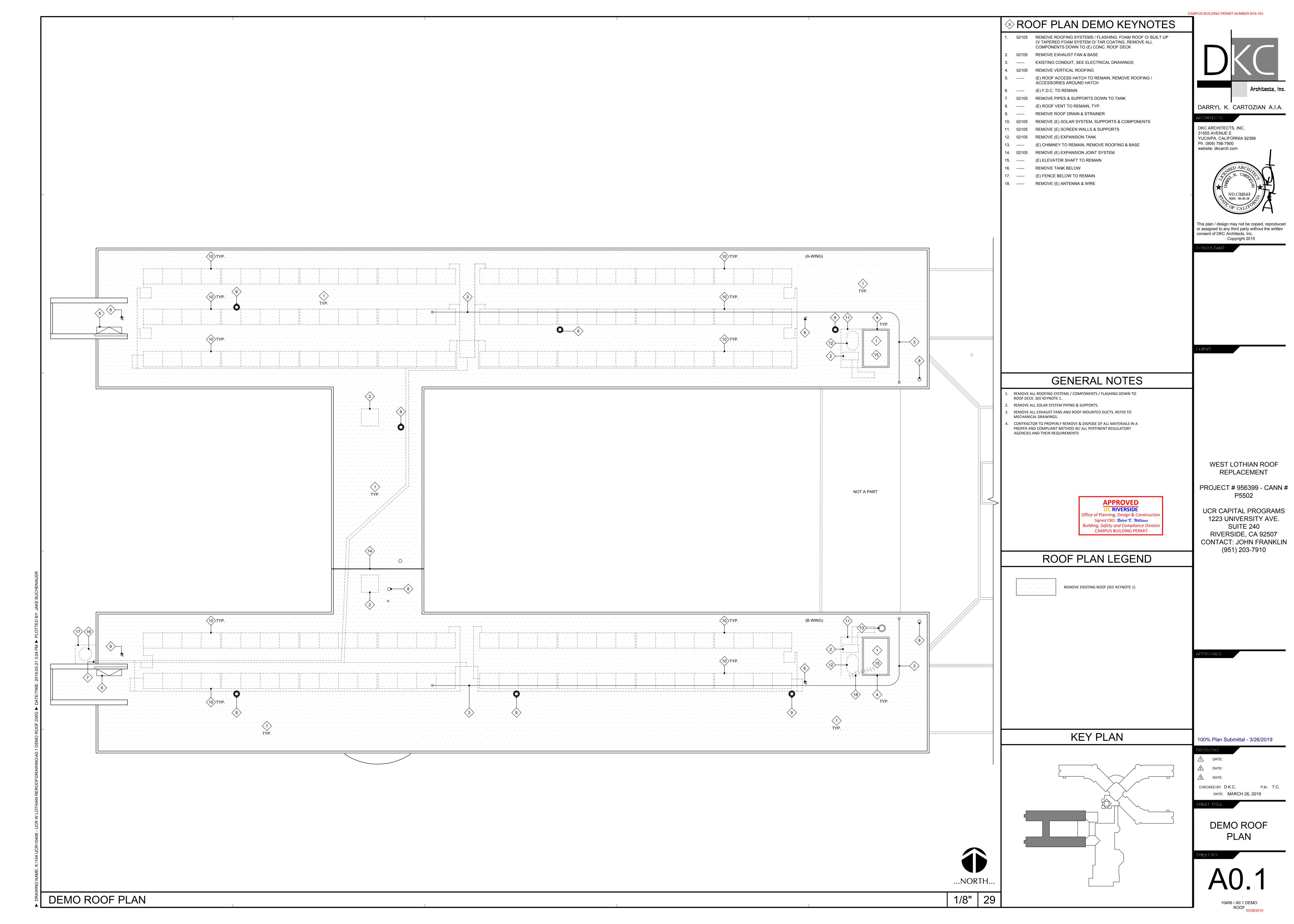
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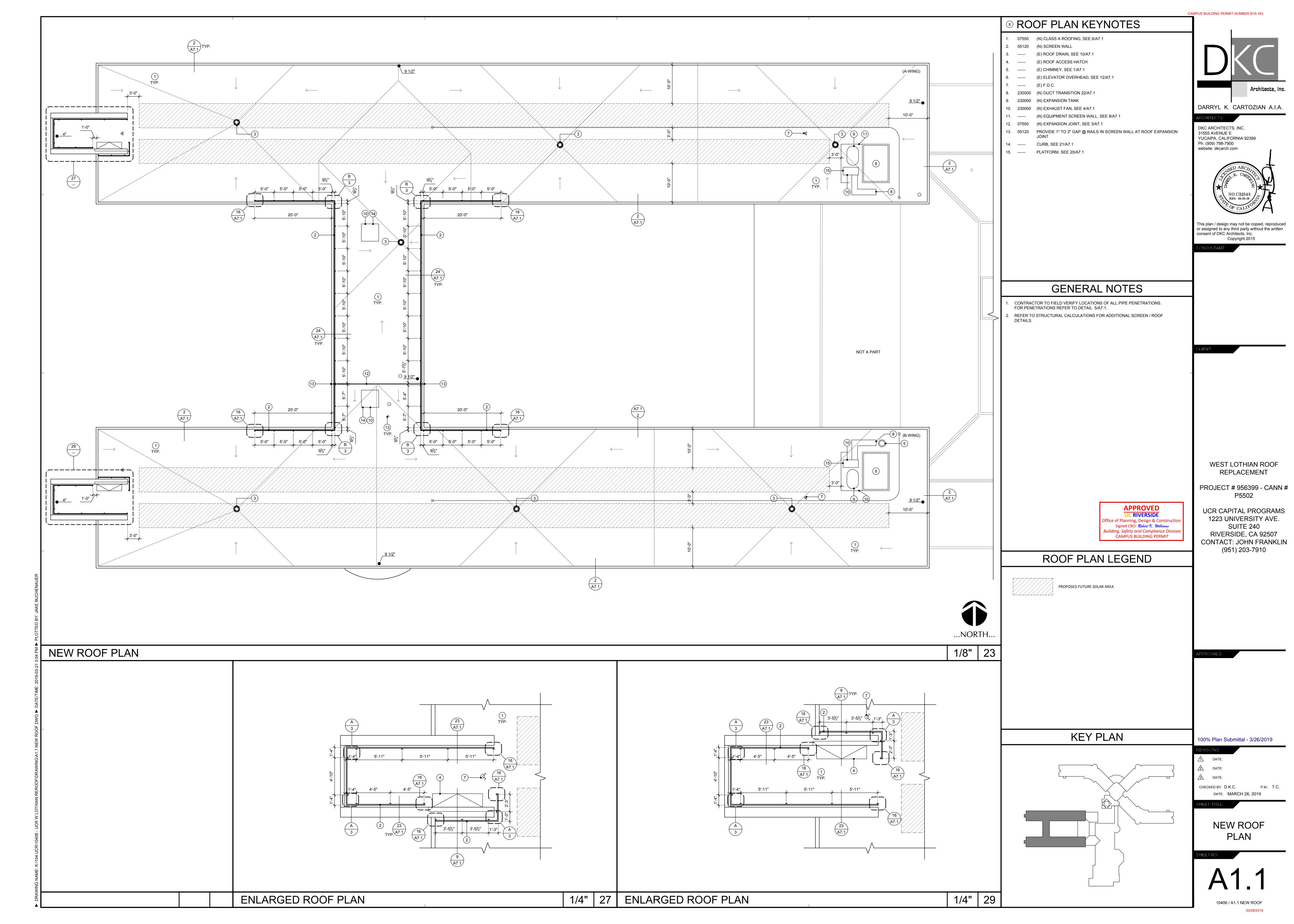
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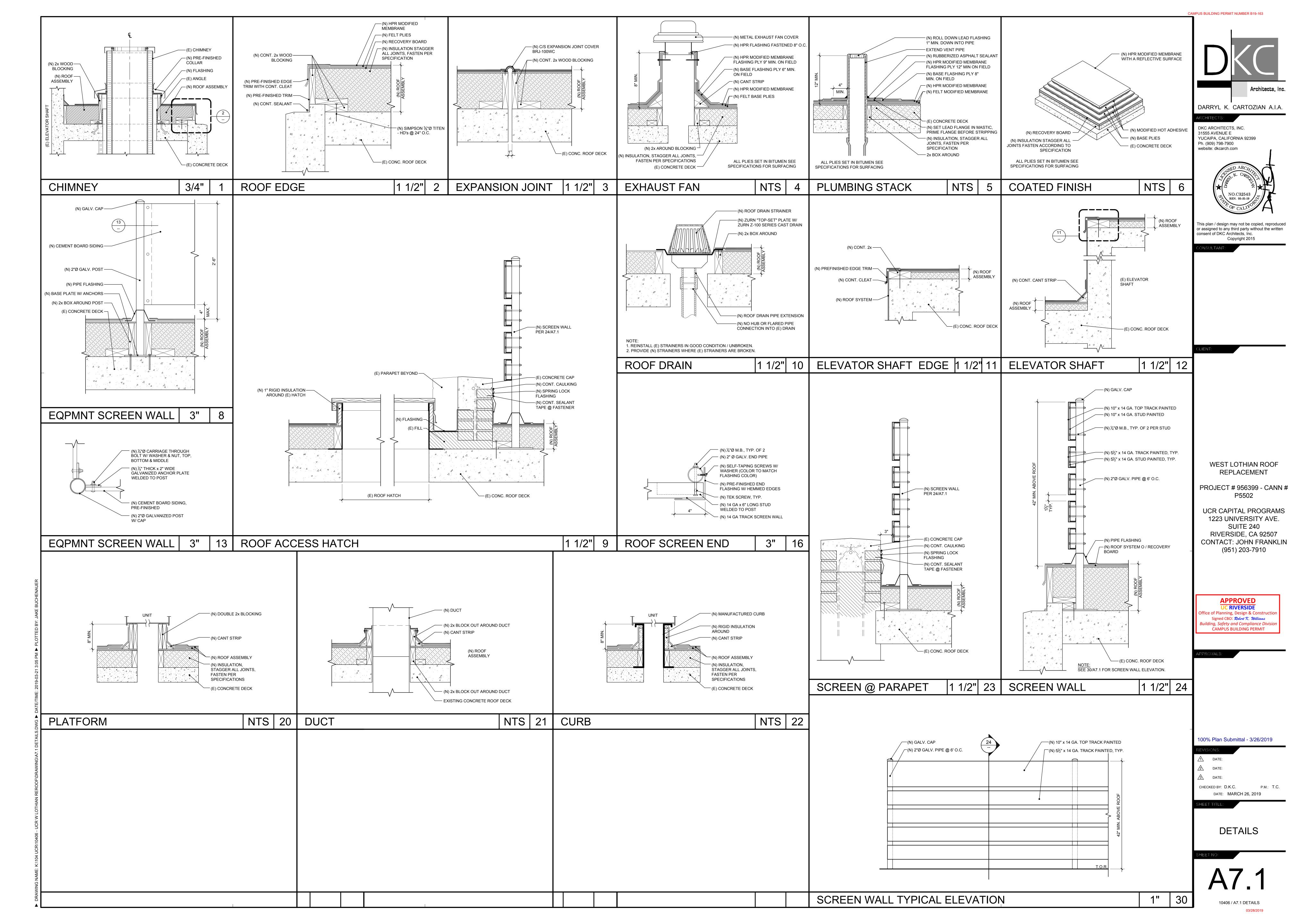
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2016 GREEN BUILDING **STANDARDS**



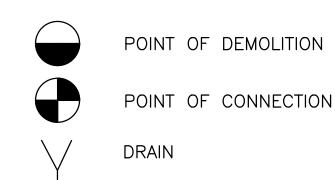






INCH

GENERAL SYMBOLS

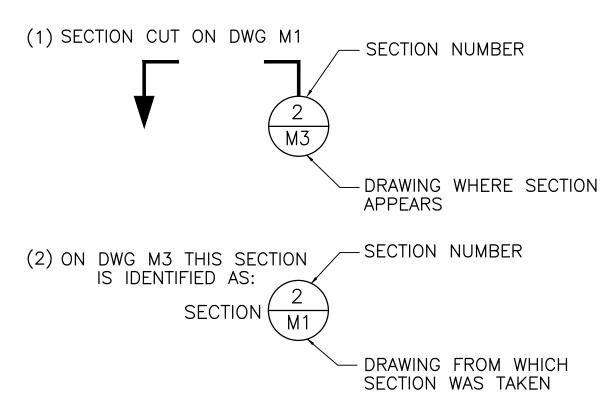


EQUIPMENT DESIGNATIONS



SECTION AND DETAIL DESIGNATIONS

E-1 ← EXHAUST GRILLE 80 CFM



(3) DETAILS ARE CROSS REFERENCED IN A SIMILAR MANNER, EXCEPT THAT DETAILS ARE IDENTIFIED BY LETTER RATHER THAN NUMBER

GENERAL MECHANICAL NOTES

- 1. PERFORM CONSTRUCTION SUCH THAT ADJACENT SPACES ARE NOT AFFECTED. ADJACENT LABS MUST REMAIN IN OPERATION THROUGHOUT CONSTRUCTION. ANY ACTIVITIES WHICH AFFECT ADJACENT LAB OPERATIONS SHALL BE COORDINATED WITH AND APPROVED BY THE UNIVERSITY REPRESENTATIVE BEFORE CONSTRUCTION BEGINS.
- 2. ALL CORRIDORS AND ROOMS WITHIN THE SCOPE OF THE PROJECT SHALL BE AIR BALANCED TO AIR FLOWS NOTED ON DRAWINGS.

MECHANICAL INDEX

HEATING, VENTILATION AND AIR CONDITIONING LEGEND

SYMBOL	DESCRIPTION
AH/AHU	AIR HANDLING UNIT
CFM	CUBIC FEET PER MINUTE
M	DAMPER ACTUATOR
	DUCT CARRYING RETURN AIR
$\boxtimes lackbox{lack} \otimes lackbox{lack}$	DUCT CARRYING SUPPLY AIR
	DUCT CARRYING EXHAUST AIR
EA	EXHAUST AIR
EF	EXHAUST FAN
	EXHAUST GRILLE
	FLEXIBLE CONNECTION
	MANUAL VOLUME DAMPER
OSA	OUTSIDE AIR
RA	RETURN AIR
RG 🔀	RETURN GRILLE
SA	SUPPLY AIR
SD	SMOKE DETECTOR

STANDARD MECHANICAL VALVE SYMBOLS

SINGLE	LINE	DOUBLE LINE
	BUTTERFLY VALVE	
	PLUG VALVE	
	GATE VALVE	
	GATE, ANGLE	
	GLOBE	
	GLOBE, ANGLE	
	BALL VALVE	
	CHECK VALVE	
	THREE WAY	
	DIAPHRAGM	
	NEEDLE	
PRV-1	PRESSURE REDUCING (NUMBER AND SPECIFY)	
	RELIEF(R) OR SAFETY(S)	
	SOLENOID	
$- \stackrel{\top}{\nabla} _{-}$	BALANCING VALVE	

LINE TYPES

	NEW PIPIN	IG OR EQUIP	MENT	
	EXISTING	(SCREENED	BACKGF	ROUND)
	EXISTING	HIDDEN (SC	REENED	BACKGROUN
<i></i>	PIPING OF	R EQUIPMENT	TO BE	DEMOLISHED

STANDARD PIPING SPECIALTY SYMBOLS

	AV			
AIR, VENT, AUTOMATIC		—— ——	UNION	
AIR VENT, MANUAL			CONCENTRIC REDUCER	
AIR SEPARATOR	(s)	 	FLEXIBLE METAL HOSE	2
FLEXIBLE CONNECTOR	—KXX	• 	ELBOW UP	
PUMP	—	C+	ELBOW DOWN	
PUMP SUCTION DIFFUSER	PSD	-+•+	TEE UP	
STRAINER	+ +	+=+	TEE DOWN	
STRAINER, BLOW OFF	+ +	-+	LATERAL UP	
·	₹	- C -	LATERAL DOWN	
THERMOMETER	<u> </u>			
TUEDVOVETED WELL ONLY	$_{ au}$ TW			

STANDARD MECHANICAL FITTING SYMBOLS

APPROVED

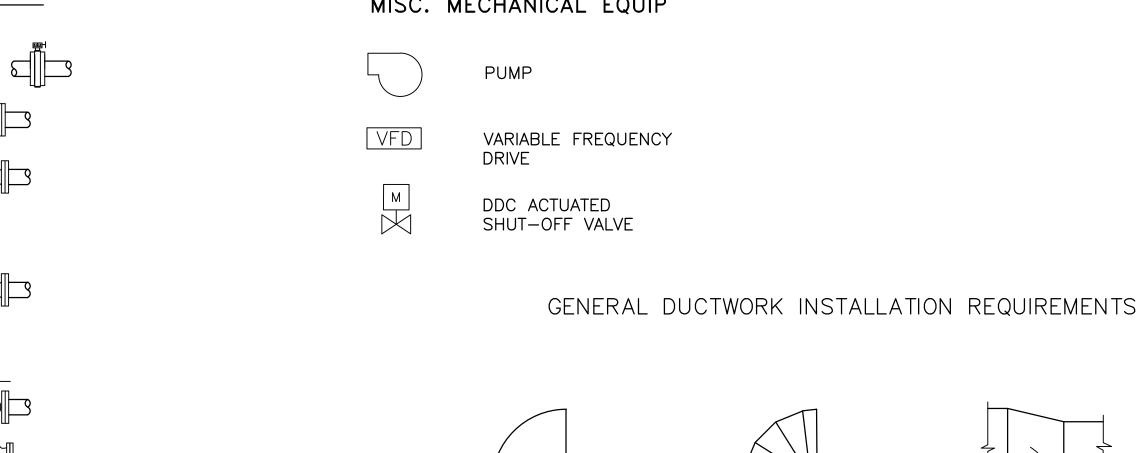
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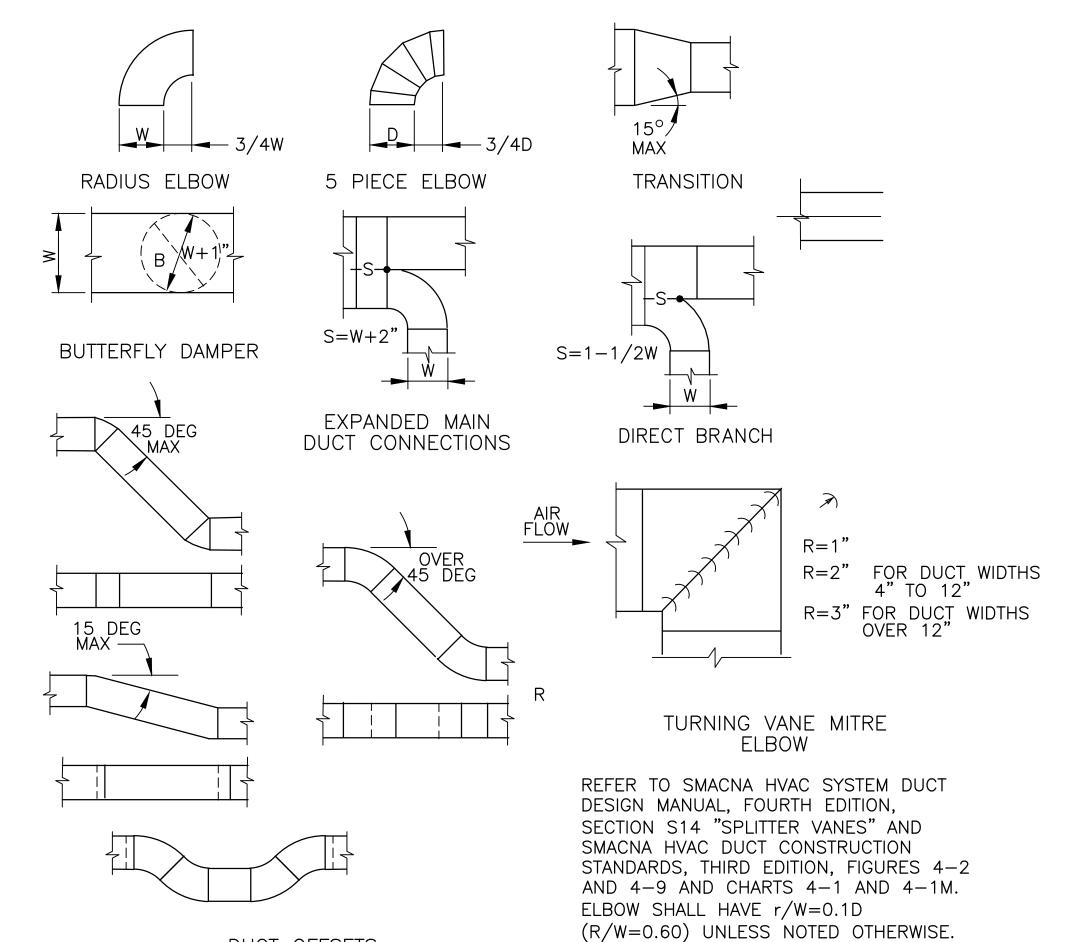
Building, Safety and Compliance Divis

CAMPUS BUILDING PERMIT

MISC. MECHANICAL EQUIP



THERMOMETER WELL, ONLY



DUCT OFFSETS

Architects, Inc.

DARRYL K. CARTOZIAN A.I.A.

DKC ARCHITECTS, INC. 31555 AVENUE E YUCAIPA, CALIFORNIA 92399 Ph. (909) 798-7900 website: dkcarch.com

CAMPUS BUILDING PERMIT NUMBER B19-163

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WEST LOTHIAN ROOF REPLACEMENT

PROJECT # 956399 - CANN # P5502

UCR CAPITAL PROGRAMS 1223 UNIVERSITY AVE. SUITE 240 RIVERSIDE, CA 92507 CONTACT: JOHN FRANKLIN (951) 203-7910

100% Plan Submittal - 3/26/2019

CHECKED BY: R.M. P.M.: N.D.U. DATE: MARCH 26, 2019

SYMBOLS, DESIGNATION AND **ABBREVIATIONS**

XHAUST FAN SO	CHEDULE													
EQUIPMENT TAG	AREA SERVED	LOCATION	MANUFACTURER	MODEL	TYPE	TYPE CFM EXTERNAL SP (IN. WC.)		TYPE CFM			MOTOR		OPERATING	NOTES
							(IN. VVC.)	HP	VOLTS	PHASE	WEIGHT (LBS)			
EF-1A	RESTROOM EXHAUST	WEST LOTHIAN ROOF	GREENHECK	CUBE 180-VGD-20	BELT DRIVE	4,500	0.50	1 1/2	208	3	150	1,2,4		
EF-2A	RESTROOM EXHAUST	WEST LOTHIAN ROOF	GREENHECK	CUBE 180-VGD-20	BELT DRIVE	4,500	0.50	1 1/2	208	3	150	1,2,4		
EF-3A	RESTROOM EXHAUST	WEST LOTHIAN ROOF	GREENHECK	USFD-118-BI	BELT DRIVE	2,200	0.50	1	208	3	200	1,3,4		
EF-4A	RESTROOM EXHAUST	WEST LOTHIAN ROOF	GREENHECK	USFD-118-BI	BELT DRIVE	2,200	0.50	1	208	3	200	1,3,4		
OTES:														
1 PROVIDE FAC	TORY MOUNTED DI	SCONNECT												

PROVIDE FACTORY ROOF CURB, PITCHED TO MATCH ROOF SLOPE. CONTRACTOR SHALL COORDINATE EXACT ROOF OPENING WITH MANUFACTURER AND PROVIDE AN ADAPTER CURB AS NECESSARY

PROVIDE FACTORY GALVANIZED BIRDSCREEN

4 PROVIDE FACTORY BACKDRAFT DAMPER

3 REFER TO SPECIFICATIONS 23 2113 FOR ADDITIONAL INFORMATION

EQUIPMENT TAG	SERVICE	LOCATION	MANUFACTURER	MODEL	TANK SIZE (GAL)	SYSTEM CONNECTION (IN)	MINIMUM PRESSURE (PSIG)	MAXIMUM PRESSURE (125 PSIG)	DIAMETER (IN)	HEIGHT (IN)	OPERATING WEIGHT (LBS)	NOTES
ET-1	CHW SYSTEM	WEST LOTHIAN ROOF	BELL AND GOSSETT	B-400	100	1	10	125	24	65	1,175	1,2,3
ET-2	CHW SYSTEM	WEST LOTHIAN ROOF	BELL AND GOSSETT	B-400	100	1	10	125	24	65	1,175	1,2,3
OTES:			1		,						1	
1 PROVIDE CAL	IFORNIA CODE SIG	HT-GLASS AND SE	ISMIC CLIPS									

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UC RIVERSIDE
Office of Planning, Design & Construction
Signed CBO: Robert X. Williams
Building, Safety and Compliance Division
CAMPUS BUILDING PERMIT

Architects, Inc.

DARRYL K. CARTOZIAN A.I.A.

CAMPUS BUILDING PERMIT NUMBER B19-163

ARCHITECTS:

DKC ARCHITECTS, INC.
31555 AVENUE E

YUCAIPA, CALIFORNIA 92399

Ph. (909) 798-7900 website: dkcarch.com

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WEST LOTHIAN ROOF REPLACEMENT

PROJECT # 956399 - CANN # P5502

UCR CAPITAL PROGRAMS
1223 UNIVERSITY AVE.
SUITE 240
RIVERSIDE, CA 92507
CONTACT: JOHN FRANKLIN
(951) 203-7910

APPROVAL

100% Plan Submittal - 3/26/2019

REVISIONS:

DATE:

DATE:

CHECKED BY: R.M. P.M.: N.D.U.

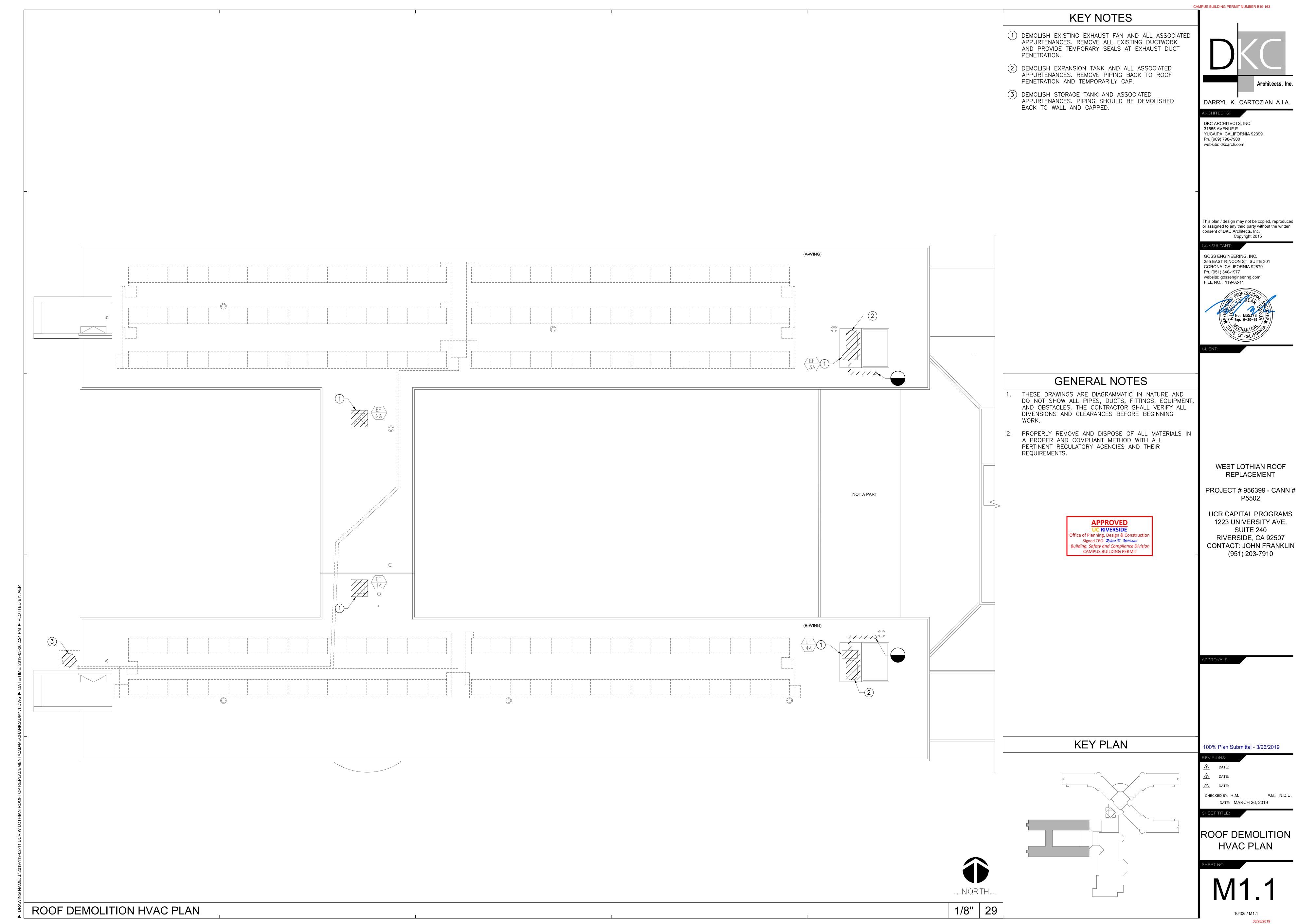
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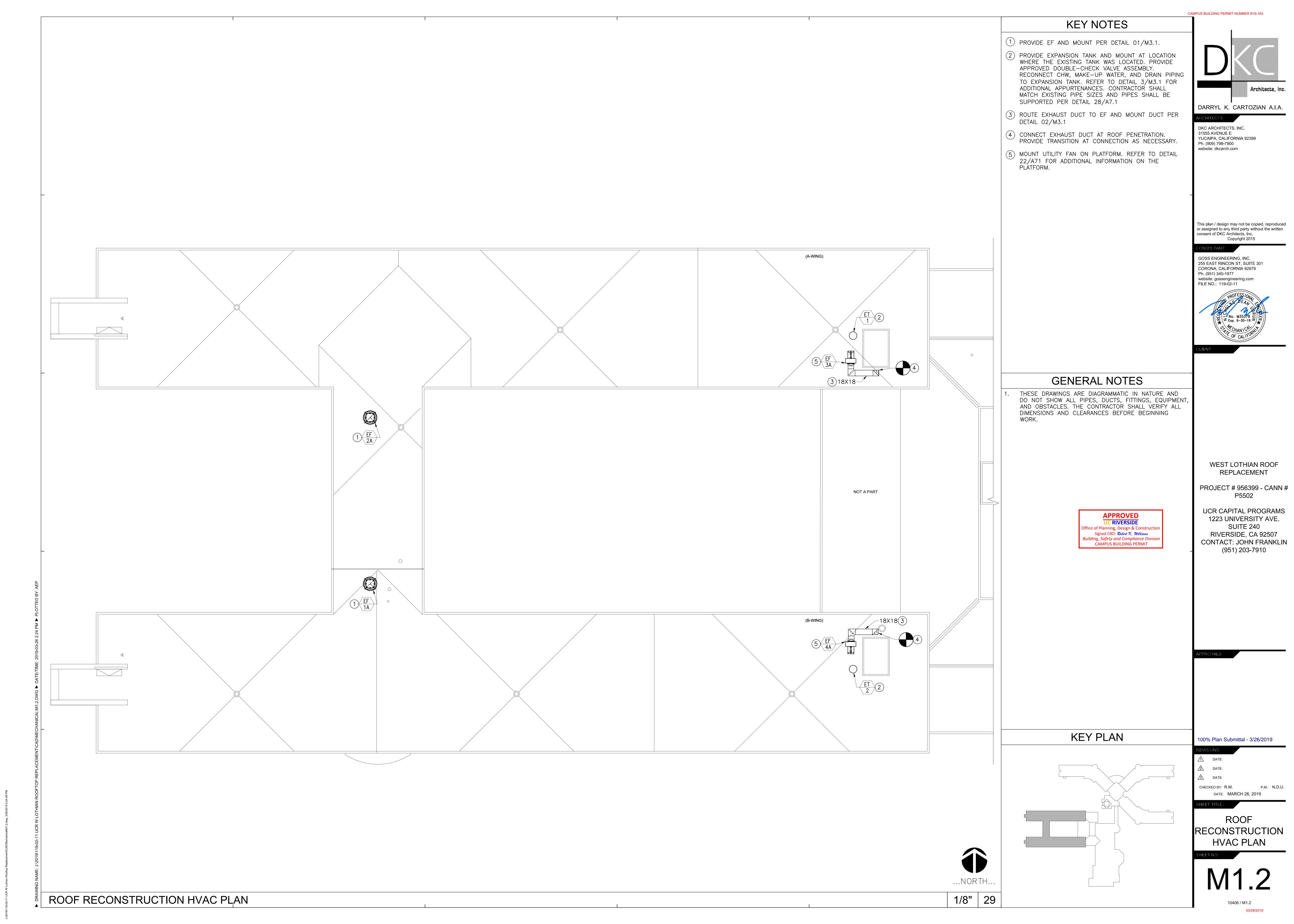
EQUIPMENT SCHEDULES

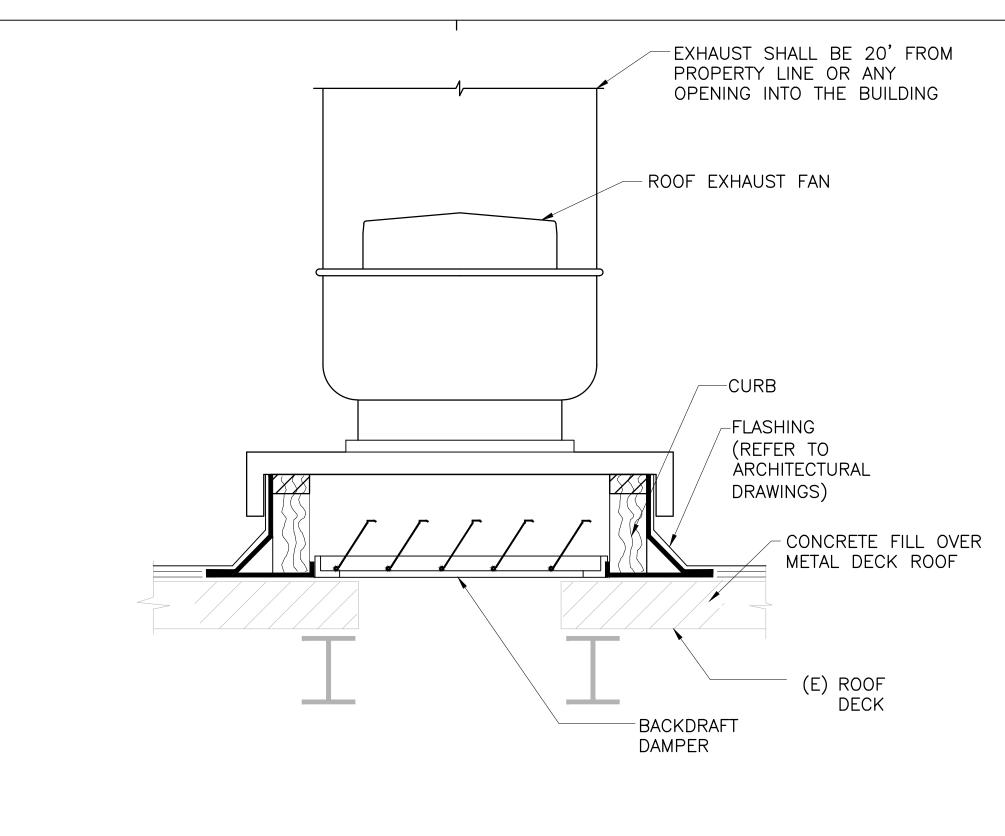
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0406 / M0.2

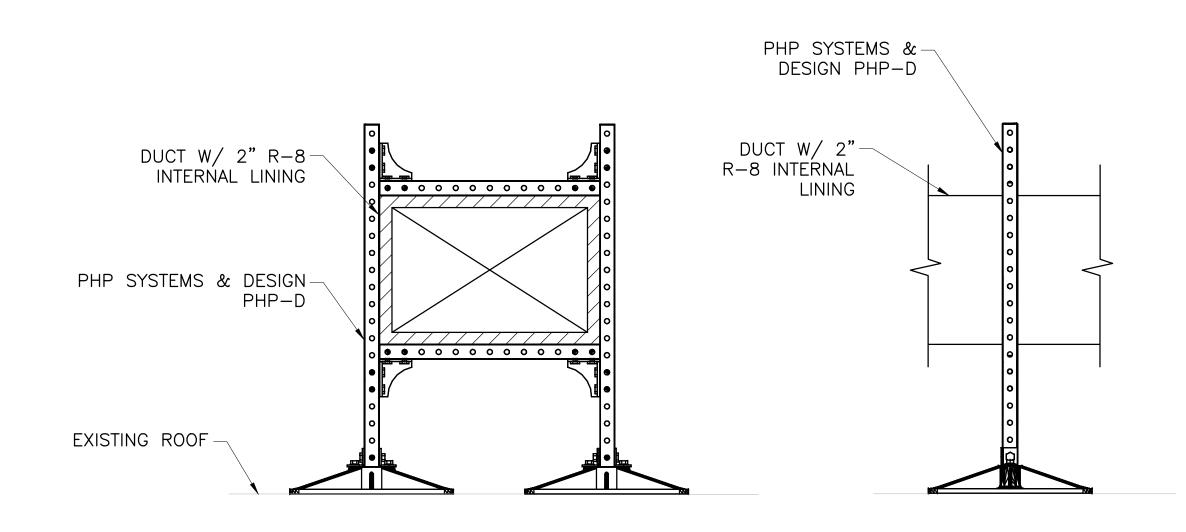
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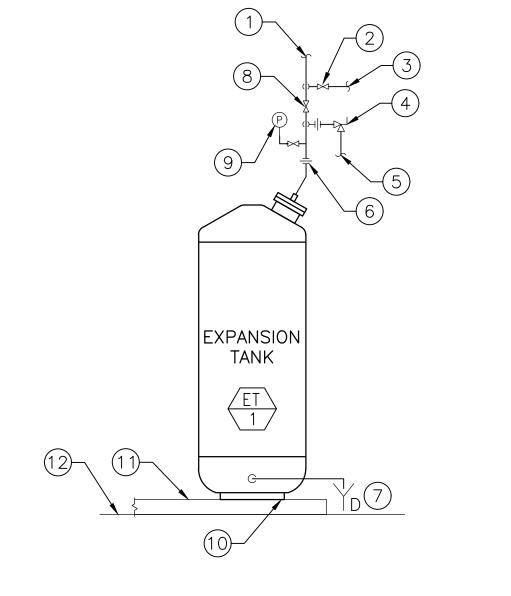




ROOF EXHAUST FAN INSTALLATION DETAIL SCALE: NONE



DUCT SUPPORT DETAIL SCALE: NONE



- (1) CONNECT TO CHW LOOP
- 2 REMOVE VALVE HANDLE AFTER PLUMBER CONNECTS MAKE-UP WATER
- 3 3/4" CONNECTION FOR MAKE-UP WATER
- 4 PRESSURE/TEMPERATURE RELIEF VALVE
- 5 PIPE TO FLOOR DRAIN
- 6 UNION (TYPICAL)
- 7 DRAIN VALVE
- 8 SHUT-OFF VALVE
- 9 PRESSURE GAUGE
- (10) ATTACHMENT TO CONCRETE
- (1) CONCRETE PAD, REFER TO STRUCTURAL
- 12) FINISH GRADE

NOTES: LOCATE ALL VALVES FOR CONVENIENT ACCESS WHEN STANDING NEXT TO TANK.

EXPANSION TANK INSTALLATION DETAIL SCALE:

CAMPUS BUILDING PERMIT NUMBER B19-163

Architects, Inc. DARRYL K. CARTOZIAN A.I.A.

DKC ARCHITECTS, INC. 31555 AVENUE E YUCAIPA, CALIFORNIA 92399 Ph. (909) 798-7900 website: dkcarch.com

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APPROVED JC RIVERSIDE Office of Planning, Design & Construction
Signed CBO: Robert K. Williams
Building, Safety and Compliance Division
CAMPUS BUILDING PERMIT

WEST LOTHIAN ROOF REPLACEMENT

PROJECT # 956399 - CANN # P5502

UCR CAPITAL PROGRAMS 1223 UNIVERSITY AVE. SUITE 240 RIVERSIDE, CA 92507 CONTACT: JOHN FRANKLIN (951) 203-7910

100% Plan Submittal - 3/26/2019

P.M.: N.D.U. CHECKED BY: R.M. DATE: MARCH 26, 2019

MECHANICAL **DETAILS**

RACEWAY OR WIRING SYSTEM ABOVE FLOOR LEVEL, CONCEALED IN WALL OR ABOVE CEILING UON

RACEWAY OR WIRING SYSTEM IN OR UNDER FLOOR OR CONCEALED IN OR BEHIND STRUCTURE OR EQUIPMENT

CONDUIT STUB ENDING WITH CAP

CIRCUITS AND RACEWAYS

JB2700A

JUNCTION BOX. OPTIONAL IDENTIFIER.

PB1035

PULL BOX. OPTIONAL IDENTIFIER.

TB1035

TERMINAL BOX. OPTIONAL IDENTIFIER.

RACEWAY SIZE WITH CONDUCTOR CONTENTS +1#1Ő AWG GRD. AND SIZES

ll DENOTES CONNECTION TO EQUIPMENT

DRAWING INDEX

SYMBOLS, DESIGNATION AND ABBREVIATIONS

ROOF DEMOLITION POWER PLAN

ROOF POWER PLAN E1.2

PANEL SCHEDULES AND DETAILS

APPLICABLE CODES

2016 CALIFORNIA BUILDING CODE (CBC) PART 2, VOLUMES 1 AND 2. TITLE 24 (BASED ON 2015 INTERNATIONAL BUILDING CODE)

2016 CALIFORNIA ELECTRIC CODE (CEC) PART 3, TITLE 24 (BASED ON 2014 NATIONAL ELECTRIC CODE)

2016 CALIFORNIA MECHANICAL CODE (CMC) PART 4, TITLE 24 (BASED ON 2015 UNIFORM MECHANICAL CODE)

2016 CALIFORNIA PLUMBING CODE (CPC) PART 5, TITLE 24 (BASED ON 2015 UNIFORM PLUMBING CODE)

2016 CALIFORNIA FIRE CODE (CFC) PART 9, TITLE 24 (BASED ON 2015 INTERNATIONAL FIRE CODE)

2016 CALIFORNIA ENERGY CODE PART 6. TITLE 24

2016 CALIFORNIA REFERENCED STANDARD CODE PART 12, TITLE 24

TITLE 19, PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS

PARTIAL LIST OF APPLICABLE NATIONAL FIRE PROTECTION ASSOCIATION (NFPA)

NFPA 13, 2016 EDITION - INSTALLATION OF SPRINKLER SYSTEMS (AMENDED BY CSFM)

NFPA 14, 2013 EDITION - INSTALLATION OF STANDPIPE AND HOSE SYSTEMS (AMENDED BY CSFM)

NFPA 17A, 2013 EDITION - WET CHEMICAL EXTINGUISHING SYSTEMS

NFPA 24, 2016 EDITION - INSTALLATION OF PRIVATE FIRE SERVICE MAINS (AMENDED BY CSFM)

NFPA 25, (2013 CALIFORNIA EDITION, BASED ON NFPA 25, 2011 EDITION) -INSPECTION, TESTING, & MAINTENANCE OF WATER-BASED FIRE PROTECTION SYSTEMS

NFPA 72, 2016 EDITION - NATIONAL FIRE ALARM AND SIGNALING CODE (AMENDED BY CSFM)

THIS LIST

NFPA 80, 2016 EDITION - FIRE DOORS AND OTHER OPENING PROTECTIVES

REFER TO CBC CHAPTER 35 FOR ADDITIONAL STANDARDS NOT PROVIDED ON

1. ELECTRICAL CONTRACTOR SHALL PERFORM ELECTRICAL INSTALLATION WORK IN CONFORMANCE WITH THE 2016 EDITION OF THE CALIFORNIA ELECTRICAL CODE (CEC) AND ALL APPLICABLE CODES, ORDINANCES, REGULATIONS AND UNIVERSITY'S STANDARDS.

GENERAL NOTES

2. CONDUIT ROUTING AND OUTLET LOCATION AS SHOWN ON THE ELECTRICAL POWER PLAN ARE DIAGRAMMATIC IN NATURE. CONTRACTOR SHALL VERIFY FEASIBILITY OF THE INSTALLATION BEFORE COMMENCING THE JOB. ANY OBSERVATIONS TO THE EXECUTION OF THE WORK SHALL BE BROUGHT TO THE ATTENTION OF THE UNIVERSITY REPRESENTATIVE IMMEDIATELY

3. PER SPECIFICATION SECTION "CONDUCTORS AND CABLES"

a. CONDUCTOR MATERIAL APPLICATIONS:

a.1 FEEDERS: COPPER. SOLID FOR NO. 10 AWG AND SMALLER; STRANDED FOR NO. 8 AWG AND LARGER;

a.2 BRANCH CIRCUITS: COPPER SOLID FOR NO. 10 AWG AND SMALLER;

STRANDED FOR NO. 8 AWG AND LARGER, b. CONDUCTOR INSULATION AND MULTICONDUCTOR CABLE APPLICATIONS AND WIRING METHODS:

b.1 BRANCH CIRCUITS CONCEALED IN CIRCUITS, WALLS AND PARTITIONS: TYPE THHN-2-THWN-2, SINGLE CONDUCTORS IN RACEWAYS. b.2 BRANCH CIRCUITS CONCEALED IN CONCRETE, BELOW

SLABS-ON-GRADE, AND UNDERGROUND: TYPE THHN-2-THWN-2, SINGLE CONDUCTORS IN RACEWAYS.

4. CONTRACTOR SHALL COORDINATE ALL WORK WITH OTHER CONSTRUCTION TRADES. CONTRACTOR SHALL NOTIFY THE OWNER'S REPRESENTATIVE OF ANY UNRESOLVED ISSUES THAT MAY DELAY INSTALLATION OF WORK.

CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING PROPER WORKING SPACE PER CALIFORNIA ELECTRICAL CODE (CEC), PARAGRAPH 110-26.

6. CONTRACTOR SHALL PROVIDE NECESSARY HARDWARE AND SUPPORTS AS REQUIRED FOR ELECTRICAL CONDUIT/WIRE NOT SCHEDULED FOR DEMOLITION PER CALIFORNIA ELECTRICAL CODE (CEC), PARAGRAPH 110-12.

7. THE SEISMIC BRACING AND ANCHORAGE OF ELECTRICAL CONDUITS, BUS DUCT, WIREWAY, AND CABLE TRAY SHALL BE IN ACCORDANCE WITH THE CALIFORNIA BUILDING CODE, CHAPTER 16 AND "GUIDELINE FOR SEISMIC RESTRAINTS OF MECHANICAL SYSTEMS AND PLUMBING PIPING SYSTEMS," PUBLISHED BY SMACNA AND PPIC, OR THE SUPERSTRUT-SEISMIC RESTRAINT SYSTEM. OR THE KIN-LINE SEISMIC RESTRAINT SYSTEM.

8. CONNECTIONS TO VIBRATING EQUIPMENT AND SEISMIC SEPARATIONS: LIQUID—TIGHT FLEXIBLE STEEL CONDUIT IN DRY INTERIOR LOCATIONS. • LIQUID-TIGHT FLEXIBLE STEEL CONDUIT IN AREAS EXPOSED TO WEATHER, DAMP LOCATIONS, CONNECTIONS TO TRANSFORMER ENCLOSURES, AND FINAL CONNECTIONS TO MOTORS.

PROVIDE A SEPARATE INSULATED EQUIPMENT GROUNDING CONDUCTOR IN FLEXIBLE CONDUIT RUNS. MAXIMUM LENGTH SHALL BE SIX FEET UNLESS OTHERWISE NOTED.

9. EQUIPMENT OUTLETS, CONDUIT, WIRE, AND CONNECTION METHODS IN HVAC AIR-PLENUMS SHALL BE APPROVED FOR USE IN PLENUMS AND SHALL CONFORM TO THE CEC.

10. ROUTE EXPOSED CONDUIT AND CONDUIT ABOVE ACCESSIBLE CEILING SPACES PARALLEL AND PERPENDICULAR TO WALLS AND ADJACENT PIPING. ARRANGE CONDUIT TO MAINTAIN HEADROOM AND TO PRESENT A NEAT

APPEARANCE. 11. WHENEVER A DISCREPANCY IN QUANTITY OR SIZE OF CONDUIT, WIRE,

EQUIPMENT DEVICES, CIRCUIT BREAKERS, GROUND FAULT PROTECTION SYSTEMS, ETC. (ALL MATERIALS), ARISES ON THE DRAWINGS OR SPECIFICATIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING AND INSTALLING ALL MATERIAL AND SERVICES REQUIRED BY THE STRICTEST CONDITIONS NOTED ON THE DRAWINGS OR IN THE SPECIFICATIONS TO ENSURE COMPLETE AND OPERABLE SYSTEMS.

Architects, Inc.

CAMPUS BUILDING PERMIT NUMBER B19-163

DARRYL K. CARTOZIAN A.I.A.

DKC ARCHITECTS, INC. 31555 AVENUE E YUCAIPA, CALIFORNIA 92399 Ph. (909) 798-7900 website: dkcarch.com

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NSULTANT:



WEST LOTHIAN ROOF REPLACEMENT

PROJECT # 956399 - CANN # P5502

UCR CAPITAL PROGRAMS 1223 UNIVERSITY AVE. SUITE 240 RIVERSIDE, CA 92507 CONTACT: JOHN FRANKLIN (951) 203-7910

APPROVED UC RIVERSIDE Office of Planning, Design & Cor

Signed CBO: Robert K. Williams

CAMPUS BUILDING PERMIT

uilding, Safety and Compliance Di

PROVALS:

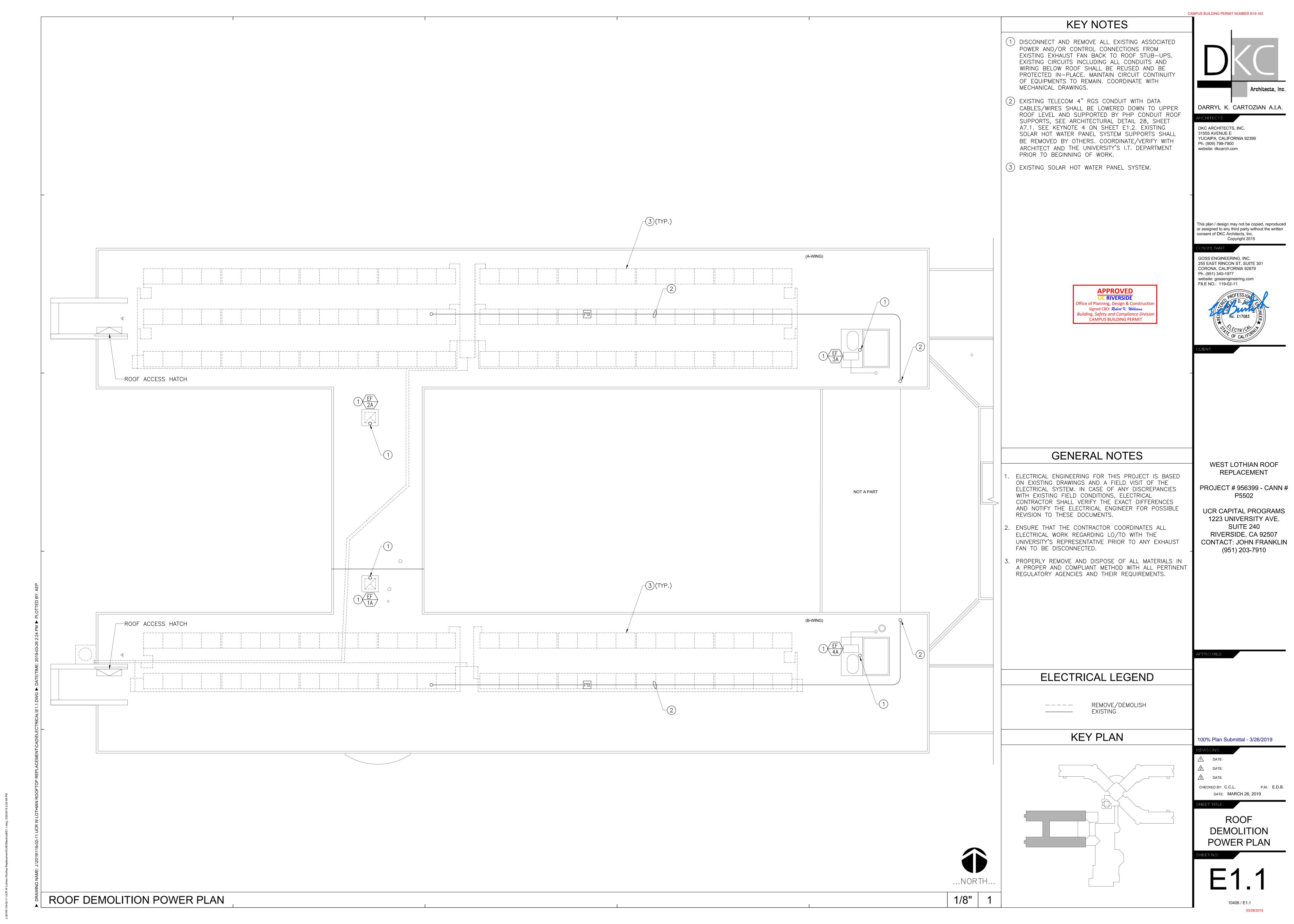
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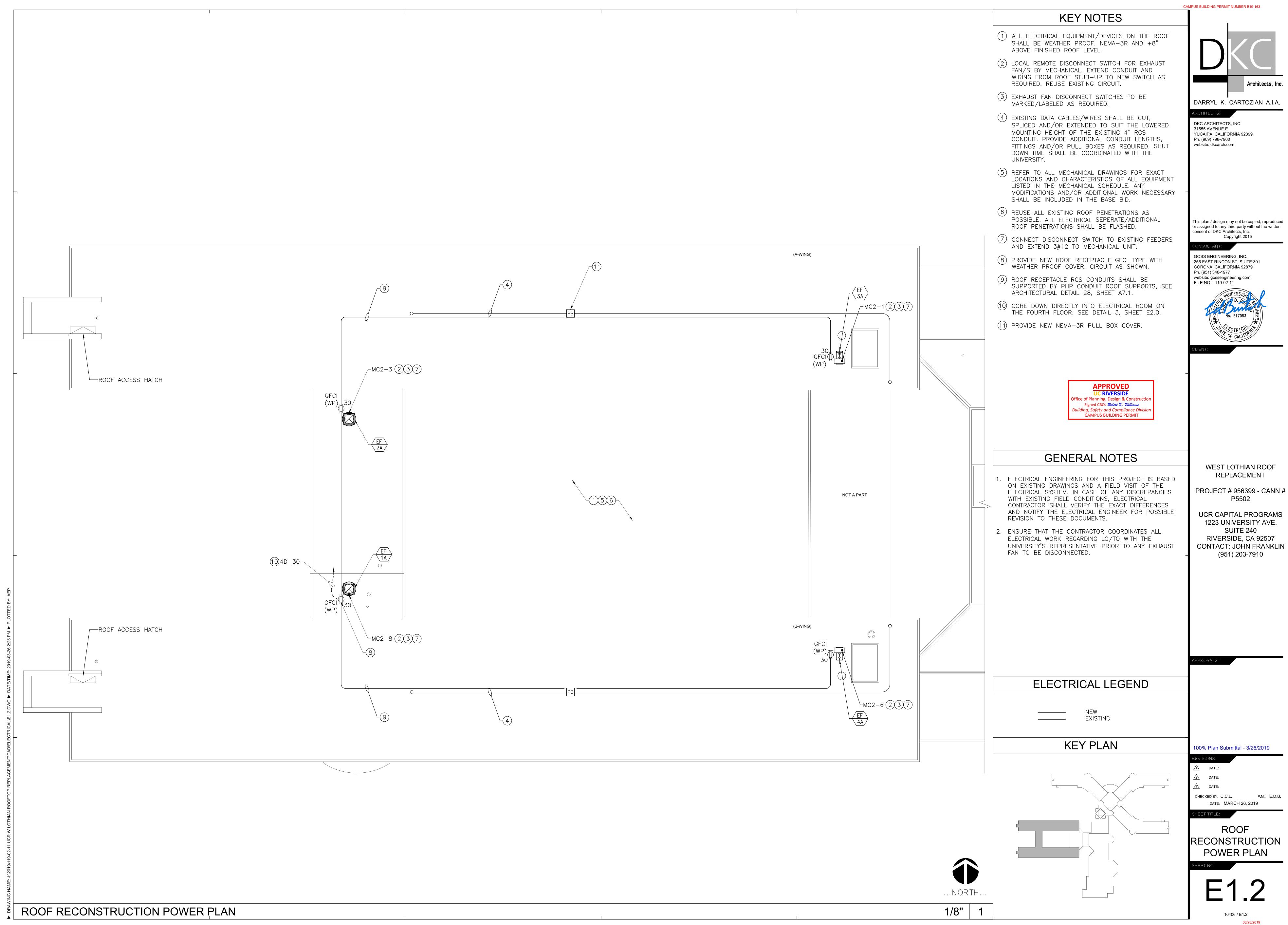
<u>∕1</u> DATE:

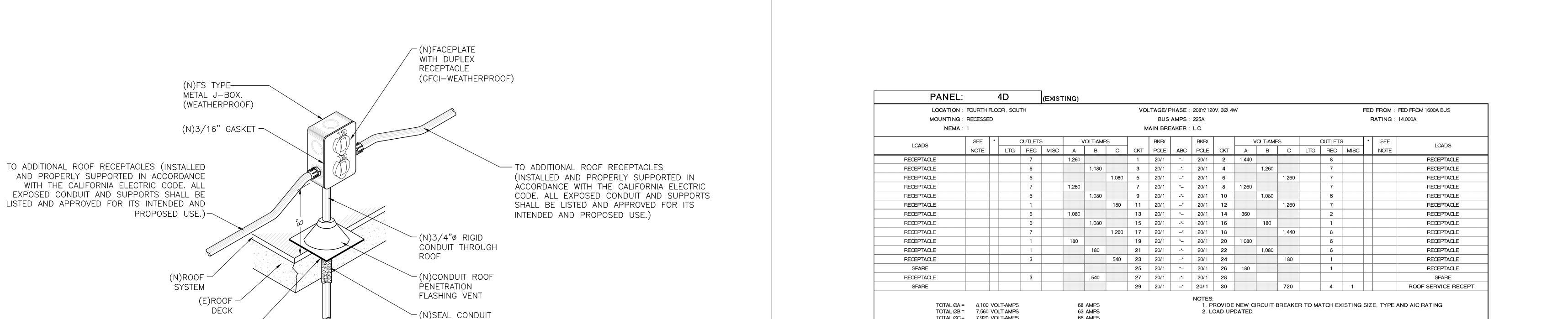
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SYMBOLS, DESIGNATION AND

ABBREVIATIONS







APPROVED UC RIVERSIDE Office of Planning, Design & Construction Signed CBO: Robert X. Williams Building, Safety and Compliance Divis CAMPUS BUILDING PERMIT

INSPECTION REQUIRED University of California, Riverside Office of Planning, Design & Construction Signed CBO: Robert K. Williams Building, Safety and Compliance Division ALL INSPECTIONS SHALL BE REQUESTED USING THE <u>CFORMS</u> SYSTEM

NONE

PROJECT # 956399 - CANN # P5502

CAMPUS BUILDING PERMIT NUMBER B19-163

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GOSS ENGINEERING, INC.

Ph. (951) 340-1977

FILE NO.: 119-02-11

255 EAST RINCON ST, SUITE 301

CORONA, CALIFORNIA 92879

website: gossengineering.com

DKC ARCHITECTS, INC. 31555 AVENUE E

Ph. (909) 798-7900

website: dkcarch.com

YUCAIPA, CALIFORNIA 92399

Architects, Inc.

UCR CAPITAL PROGRAMS 1223 UNIVERSITY AVE. SUITE 240

WEST LOTHIAN ROOF

REPLACEMENT

RIVERSIDE, CA 92507 CONTACT: JOHN FRANKLIN (951) 203-7910

100% Plan Submittal - 3/26/2019

CHECKED BY: C.C.L. Р.М.: **Е.**D.В. DATE: MARCH 26, 2019

> **PANEL** SCHEDULES AND DETAILS

NONE 2

NOT USED NONE SINGLE LINE DIAGRAM

PENETRATION WITH

3M WATER-TIGHT

DAMMING MATERIAL

FOR ROOF CONDUIT SUPPORT (PP10-C) DETAIL, REFER TO ARCHITECTURAL SHEET A7.1, DETAIL 28.

(N)FLASHING-

ROOF RECEPTACLE MOUNTING DETAIL

SEALANT

NONE

PANEL SCHEDULE

66 AMPS

66 AMPS

TOTAL ØC = 7,920 VOLT-AMPS

TOTAL PANEL = 23,580 VA @ 208V, 3Ø =

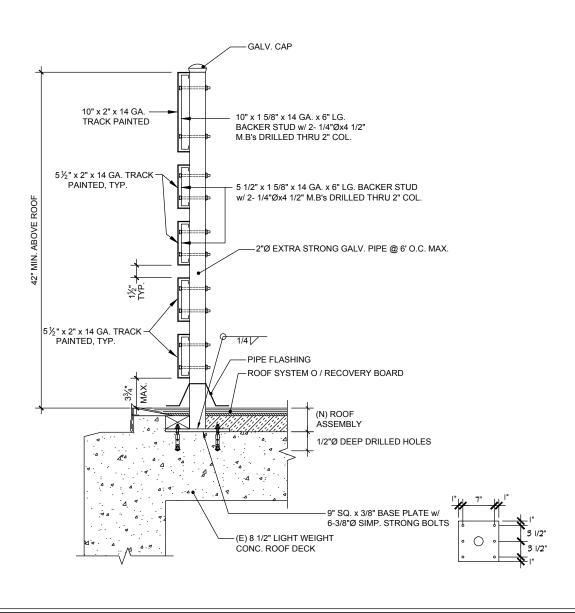
LCL = 0 VOLT-AMPS

FAN FOR HOT HALL & WATER LOUNGE PUMP 1.66KVA 9.0KVA 2.38KVA

APPROVED

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Signed CBO: Robert X. Williams
Building, Safety and Compliance Division
CAMPUS BUILDING PERMIT



Client: DKC Architects Inc.

31555 Avenue E Yucaipa, Ca. 92399

Project: West Lothian Screen Wall

UCR Capital Programs 1223 University Ave. Riverside, Ca. 92507

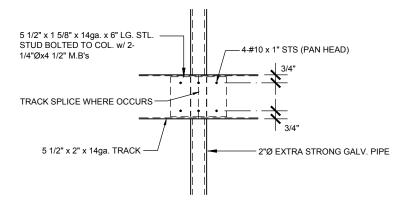
Title: Screen Wall Details

Knapp & Associates, Inc. 408 South Stoddard Avenue San Bernardino, CA. 92401 E-Mail (KNAPPAE@AOL.COM) Tel. (909) 889-0115 Fax. (909) 889-0455



UCR Project # 956399 - CANN #P5502 90% PC Submittal set 3/20/2019

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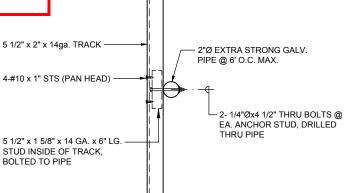


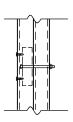
DETAIL I"=1'-O" A

APPROVED

UC RIVERSIDE

Office of Planning, Design & Construction
Signed CBO: Robert X. Williams
Building, Safety and Compliance Division
CAMPUS BUILDING PERMIT





DETAIL

|"=|'-0"

В

Client: DKC Architects Inc.

31555 Avenue E Yucaipa, Ca. 92399

Project: West Lothian Screen Wall

UCR Capital Programs 1223 University Ave. Riverside, Ca. 92507

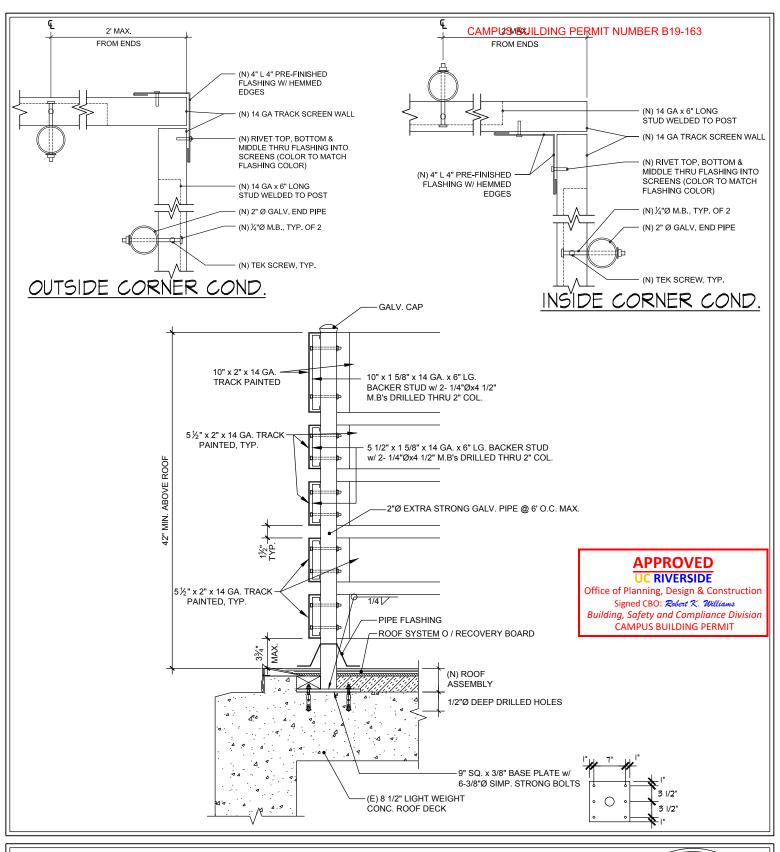
Title: Screen Wall Details

Knapp & Associates, Inc.

408 South Stoddard Avenue
San Bernardino, CA. 92401
E-Mail (KNAPPAE@AOL.COM)
Tel. (909) 889-0115
Fax. (909) 889-0455



UCR Project # 956399 - CANN #P5502 90% PC Submittal set 3/20/2019



Olient: DKC Architects Inc. 31555 Avenue E

Yucaipa, Ca. 92399

Project: West Lothian Screen Wall

UCR Capital Programs 1223 University Ave. Riverside, Ca. 92507

Title: Screen Wall Details

Knapp & Associates, Inc. 408 South Stoddard Avenue San Bernardino, CA. 92401 E-Mail (KNAPPAE@AOL.dOM) Tel. (909) 889-0115 Fax. (909) 889-0455



UCR Project # 956399 - CANN #P5502 90% PC Submittal set 3/20/2019



UCR Project # 956399 - CANN #P5502 90% PC Submittal set 3/20/2019

Structural Calculations

for

West Lothian Roof Screens

University of Riverside Riverside, Ca.



APPROVED

UC RIVERSIDE

Office of Planning, Design & Construction
Signed CBO: Robert X. Williams
Building, Safety and Compliance Division
CAMPUS BUILDING PERMIT

Date: March 12, 2019

Prepared By: LCK

Knapp & Associates Inc.

408 SOUTH STODDARD AVENUE E-mail KNAPPAE@AOL.COM TEL. (909) 889-0115 Fax (909) 889-0455

WEST LOTHIA ROOF SCREENS UNIVERSITY OF CA. ERIVERSIDE

Riverside, CA

By:

HL 03/04/19

Date:

Proj# Sheet:

WIND PRESSURE FOR SOLID FREESTANDING WALLS AND SIGNS

CHAPTER 29 OF ASCE 7-10

Risk Category 11 Table 1604.5-CBC 2013

Basic Wind Speed, V 110 mph

Wind Directionality Factor, Kd 0.85 Table 26.6-1

§ 26.7 **Exposure Category** C Topographic Factor, Kzt 1 § 26.8 Gust-effect Factor, G 0.85 § 26.9

Mean Height 36 ft

Velocity Pressure Coefficient, K_h Table 29.3-1 1.02

Velocity Pressure

 $q_h = 0.00256K_hK_{zt}K_dV^2$ = **26.75** psf

Eq. 29.3-1

VEL (ASD) = 26.75(.6) 12.00 ft

3.50 ft

36.00 ft =

SIGN / WALL

Net Force Coefficient, C_f

1.85

Figure 29.4-1

CASE A

 $P = q_h G C_f$

42.1 psf

Eq. 29.4-1

(STRENGTH LEVEL)

PASO = 42.1 (.6) = 25.26 psf

M/Post (66%) = 25.26(3.6)/2 ×6 = 982.3"#

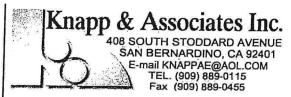
FOR 2" O EXTRAGRAGE PIPE; Fb = 11,788 PSI.

. 218 WALL THICKNESS

5,02#/LIN, FT.

M=5.5 (25.26) (6) /B=52.1 #; Sy=0.5113

Fb = 52.1(12/0,5113" = 1,222.7 psi. 52" × 2" × 14ga TRACK 15 OK



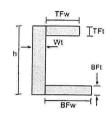
WEST LOTHIA ROSE SCREENS
CAMPUS BUILDING PERMIT NUMBER B19-163
UNIVERSITY OF CA. ERIVERSIDE By: Le

Riverside, CA

By: LCK Date: 3-19

Proj #
Sheet:

2019



Channel

h: 5

TFw: 2

TFt: 0.068

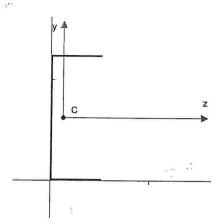
BFw: 2

BFt: 0.068

Wt: 0.068

r: 0

Free Online Moment of Inertia Calculator | SkyCiv



Units: in

Calculate

5/2" × 2" × 14 ga. TRACK

Notat	ion	Value	Unit
Α	0	0.612	in ²
Iz	0	2.362512576	in ⁴
ly	0	0.2523590311	in ⁴
Izp	0	2.362512576	in ⁴
I_{yp}	0	0.2523590311	in ⁴
αp	0	0	deg
$C_{\mathbf{z}}$	0	0.493555556	in
Cy	0	2.5	in
$Q_{\mathbf{Z}}$	0	0.547876	in ³
Qy	0	0.1612832899	in ³
S_z	0	0.9450050304	in ³
Sy	0	0.5113082575	in ³
J	0	0.000943296	in ⁴



Knapp & Associates Inc.

E-mail KNAPPAE@AOL.COM TEL. (909) 889-0115 Fax (909) 889-0455

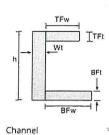
WEST LOTHIA ROOF & UNIVERSITY OF CA. ERIVERSIDE

Date: 3-19

Proi# Sheet:

ER B19-163

2019



h: 10

TFw: 2

TF#: 0.068

BFw: 2

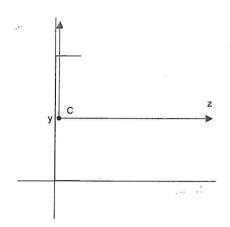
BFt: 0.068

Wt: 0.068

r: 0

Free Online Moment of Inertia Calculator | SkyCiv

Riverside, CA



Units: in

Calculate

10" × 2" × 14 ga. TRACK

Nota	tion	Value	Unit
Nota	LIOH	value	Onit
Α	0	0.952	in ²
Iz	0	12.3746059093	in ⁴
ly	0	0.2986504305	in ⁴
Izp	0	12.3746059093	in ⁴
I_{yp}	0	0.2986504305	in ⁴
α_p	0	0	deg
Cz	0	0.3294285714	in
c_{y}	0	5	In
Qz	0	1.525376	in ³
$Q_{\mathbf{y}}$	0	0.1974997273	in ³
Sz	0	2.4749211819	in ³
s_y	0	0.9065711246	in ³
J	0	0.0014673493	in ⁴

$$M = \frac{(6).833(25.26)}{8}$$

$$= 94.65^{14}$$

$$f_{5} = \frac{94.65(12)}{0.90657} = 1253 \text{ psi.}$$

$$10^{11} \times 2^{11} \times 14 \text{ ga.} \text{ TRACK 15 oK.}$$



Project Title: EngineerCAMPUS BUILDING PERMIT NUMBER B19-163 Project ID: Project Descr:

Printed: 7 MAR 2019, 3:24PM

Steel Base Plate

Lic. #: KW-06000933

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Licensee: KNAPP ARCHITECTURE ENGINEERS

Code References

Calculations per AISC Design Guide # 1, IBC 2015, CBC 2016, ASCE 7-10

Load Combination Set: ASCE 7-10

General Information

|--|

AISC Design Method

Load Resistance Factor Design

36.0 ksi

Steel Plate Fy Concrete Support f'c 3.0 ksi

Assumed Bearing Area: Full Bearing

Φ c : LRFD Resistance Factor

0.60

Nominal Bearing Fp per J8

3.40 ksi

Column & Plate

Web Thickness

Column Properties

Steel Section:	HSS2x2x1/4
Donth	

Depth Width

2 in Flange Thickness 0.233 in

in

Area lxx lyy

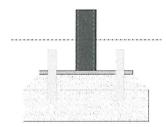
1.51 in^2 in⁴ in^4

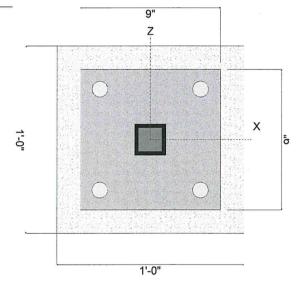
2 in

Plate Dimensions N: Length 9.0 in B: Width 9.0 in

0.3750 in **Thickness** Column assumed welded to base plate. **Support Dimensions**

Width along "X" 12.0 in Length along "Z' 12.0 in





-	
Applied	Loads

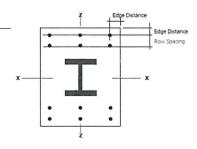
	P-Y	V-Z	M-X
D : Dead Load	k	k	k-ft
L : Live	k	k	k-ft
Lr : Roof Live	k	k	k-ft
S : Snow	k	k	k-ft
W : Wind	k	0.540 k	1.550 k-ft
E : Earthquake	k	k	k-ft
H: Lateral Earth	k	k	k-ft
" P " = Gravity load "+" sig	n ie downward	"+" Momente create higher e	oil pressure at +7 edge

= Gravity load, "+" sign is downward. Moments create higher soil pressure at +Z edge. "+" Shears push plate towards +Z edge.

+Mx

Anchor Bolts

Anchor Bolt or Rod Description 0.375" Max of Tension or Pullout Capacity..... k k Shear Capacity..... Edge distance : bolt to plate..... 1.250 in Number of Bolts in each Row..... 2.0 Number of Bolt Rows..... 1.0



Project Title: EngineerCAMPUS BUILDING PERMIT NUMBER B19-163 Project ID: Project Descr:

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Bearing Stress OK

Steel Base Plate

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GOVERNING DESIGN LOAD	O CASE SUMMARY	Mu : Max. Moment	0.974 k-in
Plate Design Summary		fb : Max. Bending Stress	27.712 ksi
Design Method	Load Resistance Factor Design	Fb : Allowable :	32.400 ksi
Governing Load Combination Governing Load Case Type Governing STRESS RATIO	+1.20D+1.60Lr+0.50W+1.60H Axial + Moment, L/2 < Eccentricity, Tension on 1.0	Fy * Phi Bending Stress Ratio	0.855 Bending Stress OK
Design Plate Size	9" x 9" x 0 -3/8"	fu : Max. Plate Bearing Stress	2.040 ksi
Pu : Axial Mu : Moment	0.000 k 0.000 k-ft	Fp : Allowable :	2.040 ksi
Mu . Moment	0.000 K-IL	Bearing Stress Ratio	1.000

Load Comb. : +1.40D+1.60H			Axial Load Only, No Moment
Loading Pu: Axial Design Plate Height Design Plate Width Will be different from entry if partial bearing used.	0.000 k 9.000 in 9.000 in	Bearing Stresses Fp : Allowable fu : Max. Bearing Pressure Stress Ratio Plate Bending Stresses	2.040 ksi 0.000 ksi 0.000
A1 : Plate Area A2: Support Area sqrt(A2/A1)	81.000 in^2 144.000 in^2 1.333	Mmax = Fu * L^2 / 2 fb : Actual Fb : Allowable Stress Ratio	0.000 k-in 0.000 ksi 32.400 ksi 0.000
Distance for Moment Calculation " m "	3.550 in 3.550 in 0.000 in^2 0.000 0.000 in 0.000 in 3.550 in		
Load Comb. : +1.20D+0.50Lr+1.			Axial Load Only, No Moment
	JULT 1.00H		Axiai Load Olliy, No Molliell
Loading Pu : Axial Design Plate Height Design Plate Width Will be different from entry if partial bearing used.	0.000 k 9.000 in 9.000 in	Bearing Stresses Fp : Allowable fu : Max. Bearing Pressure Stress Ratio Plate Bending Stresses	2.040 ksi 0.000 ksi 0.000
Loading Pu : Axial Design Plate Height Design Plate Width	0.000 k 9.000 in	Fp : Allowablefu : Max. Bearing Pressure	2.040 ksi 0.000 ksi

Project Title: EngineerCAMPUS BUILDING PERMIT NUMBER B19-163 Project ID: Project Descr:

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Steel Base Plate

Lic. # : KW-06000933

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Load Comb.: +1.20D+1.60L+0.5	50S+1.60H		Axial Load Only, No Moment
Loading	0.000 L	Bearing Stresses	0.040 (**)
Pu : Axial	0.000 k	Fp : Allowable	2.040 ksi
Design Plate Height	9.000 in	fu : Max. Bearing Pressure	0.000 ksi
Design Plate Width	9.000 in	Stress Ratio	0.000
Will be different from entry if partial bearing used.		Plate Bending Stresses	2 222 1 :
A1 : Plate Area	81.000 in^2	Mmax = Fu * L^2 / 2	0.000 k-in
A2: Support Area	144.000 in^2	fb : Actual	0.000 ksi
sqrt(A2/A1)	1.333	Fb : Allowable	32.400 ksi
		Stress Ratio	0.000
Distance for Moment Calculation			
" m "	3.550 in		
" n "	3.550 in		
X	0.000 in^2		
Lambda	0.000		
n'	0.000 in		
n' * Lambda	0.000 in		
L = max(m, n, n")	3.550 in		
Load Comb. : +1.20D+1.60Lr+0.	50L+1.60H		Axial Load Only, No Moment
Loading		Bearing Stresses	
Pu : Axial	0.000 k	Fp : Allowable	2.040 ksi
Design Plate Height	9.000 in	fu : Max. Bearing Pressure	0.000 ksi
Design Plate Width	9.000 in	Stress Ratio	0.000
Will be different from entry if partial bearing used.	202 20 11	Plate Bending Stresses	
A1 : Plate Area	81.000 in^2	Mmax = Fu * L^2 / 2	0.000 k-in
A2: Support Area	144.000 in^2	fb : Actual	0.000 ksi
		Fb : Allowable	32.400 ksi
sqrt(A2/A1)	1.333	Stress Ratio	0.000
Distance for Moment Calculation			
" m "	3.550 in		
" n "	3.550 in		
X	0.000 in^2		
Lambda	0.000		
n'	0.000 in		
n' * Lambda	0.000 in		
L = max(m, n, n")	3.550 in		
Load Comb.: +1.20D+1.60Lr+0.	50W+1.60H	Ax	kial Load + Moment, Ecc. > L/2
Loading		Calculate plate moment from bolt tension	
<u>Loading</u> Pu : Axial	0.000 k	Tension per Bolt	0.603 k
Mu : Moment	0.775 k-ft	Tension : Allowable	0.000 k
Eccentricity	93,000.000 in	Stress Ratio	0.000 k
The Control of the Co	81.000 in^2	Oli COS Ratio Illininininini	0.000
A1 : Plate Area	144.000 in^2	Dist. from Bolt to Col. Edge	2.300 in
A2 : Support Area	144.000 11112	Effective Bolt Width for Bending	9.000 in
sqrt(A2/A1)	1.333	Plate Moment from Bolt Tension	0.308 k-in
Calculate plate moment from bearing			
max(m, n)	3.700 in	Bearing Stresses	
"A" : Bearing Length	0.131 in	Fp : Allowable	2.040 ksi
Mpl : Plate Moment	0.041 k-in	fu : Max. Bearing Pressure	(set equal to Fp)
And Angelow strategies appropriately as		Stress Ratio	1.000
		Plate Bending Stresses	
		Mmax	0.490 k-in
		fb : Actual	13.946 ksi
		Fb : Allowable	32.400 ksi
		Stress Ratio	0.430
			endered.

Project Title: EngineerCAMPUS BUILDING PERMIT NUMBER B19-163 Project ID: Project Descr:

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Load Comb. : +1.20D+1.60Lr-0.	50W+1.60H		Axial Load + Moment, Ecc. > L/2
Loading		Calculate plate moment from bolt tension	
Pu : Axial	0.000 k	Tension per Bolt	0.603 k
Mu: Moment	0.775 k-ft	Tension : Allowable	0.000 k
Eccentricity	93,000.000 in	Stress Ratio	0.000
A1 : Plate Area	81.000 in^2		
A2 : Support Area	144.000 in^2	Dist. from Bolt to Col. Edge	2.300 in
sqrt(A2/A1)	1.333	Effective Bolt Width for Bending	9.000 in
oqia(/azi/ii /	1.000	Plate Moment from Bolt Tension	0.308 k-in
Calculate plate moment from bearing			
max(m, n)	3.700 in	Bearing Stresses	2222
"A" : Bearing Length	0.131 in	Fp : Allowable	2.040 ksi
Mpl : Plate Moment	0.041 k-in	fu : Max. Bearing Pressure	(set equal to Fp)
		Stress Ratio	1.000
		Plate Bending Stresses	
		Mmax	0.490 k-in
		fb : Actual	13.946 ksi
		Fb : Allowable	32.400 ksi
		Stress Ratio	0.430
Load Comb.: +1.20D+0.50L+1.6	60S+1.60H		Axial Load Only, No Moment
Loading		Bearing Stresses	
Pu : Axial	0.000 k	Fp : Allowable	2.040 ksi
Design Plate Height	9.000 in	fu : Max. Bearing Pressure	0.000 ksi
Design Plate Width	9.000 in	Stress Ratio	0.000
Will be different from entry if partial bearing used.		Plate Bending Stresses	
A1 : Plate Area	81.000 in^2	Mmax = Fu * L^2 / 2	0.000 k-in
A2: Support Area	144.000 in^2	fb : Actual	0.000 ksi
sgrt(A2/A1)	1.333	Fb : Allowable	32.400 ksi
Sqit(AZIAT)	1.000	Stress Ratio	0.000
Distance for Moment Calculation			
" m "	3.550 in		
" n "	3.550 in		
X	0.000 in^2		
Lambda	0.000		
n'	0.000 in		
n' * Lambda	0.000 in		
L = max(m, n, n")	3.550 in		
Load Comb. : +1.20D+1.60S+0.	50W+1.60H		Axial Load + Moment, Ecc. > L/2
<u>Loading</u>		Calculate plate moment from bolt tension	
Pu : Axial	0.000 k	Tension per Bolt	0.603 k
Mu : Moment	0.775 k-ft	Tension : Allowable	0.000 k
Eccentricity	93,000.000 in	Stress Ratio	0.000
A1 : Plate Area	81.000 in^2	Dist 6 D. 11 - O. I. E. I.	0.000 1
A2 : Support Area	144.000 in^2	Dist. from Bolt to Col. Edge	2.300 in
sqrt(A2/A1)	1.333	Effective Bolt Width for Bending Plate Moment from Bolt Tension	9.000 in 0.308 k-in
Calculate plate moment from bearing		r late Moment from Doit Tension	0.000 min
max(m, n)	3.700 in	Bearing Stresses	
"A" : Bearing Length	0.131 in	Fp : Allowable	2.040 ksi
Mpl : Plate Moment	0.041 k-in	fu : Max. Bearing Pressure	(set equal to Fp)
mge i i iana mamana	1 W W	Stress Ratio	1.000
		Plate Bending Stresses	
		Mmax	0.490 k-in
		fb : Actual	13.946 ksi
		Fb : Allowable	32.400 ksi
		O4 D 4'	0.400

0.430

Stress Ratio

Project Title:
Engineer:CAMPUS BUILDING PERMIT NUMBER B19-163
Project ID:
Project Descr:

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Calculate plate moment from bott tension Calculate plate moment from bearing Calculate plate moment from bott tension Calculate plate moment from bearing Calculate plate moment from bott tension Calculate plate moment from bott tension Calculate plate moment from bott tension Calculate plate moment from bearing Calculate plate moment from bott tension Calculate plate moment from bott tension Calculate plate moment from bearing Calculate plate moment from bott tension Calculate plate moment from bott tension Calculate plate moment from bearing Calculate plate moment from bott tension Calculate plate moment from	Load Comb. : +1.20D+1.60S-0.	50W+1.60H		Axial Load + Moment, Ecc. > L/2
Mu : Moment 0.775 krt Tension : Allowable 0.000 k A1 : Plate Area 81.000 in²2 Stress Ratio 0.000 k A2 : Support Area 144.000 in²2 sqrt(A2/A1) 1.333 Dist. from Bolt to Col. Edge 2.300 in Effective Bolt Width for Bending 9,000 in 9,000 in max(m, n) 3.700 in Bearing Stresses 2.040 ksi *A* : Bearing Length 0.131 in Fp : Allowable 2.040 ksi Mpl : Plate Moment 0.001 kin Stress Ratio 0.490 kin Mpl : Plate Moment 0.041 kin fu : Max. Bearing Pressure (set equal to Fp) Stress Ratio 1.000 1.000 Plate Bending Stresses Mmax 0.490 kin fb : Actual 13.946 ksi Fb : Allowable 32.400 ksi Stress Ratio 0.430 Loading Pu : Axial 1.214 k Pu : Axial 0.000 k 1.214 k Mu : Moment 1.550 krf Tension per Bot 1.214 k Tension per Bot 1.214 k Tension per Bot 0.000 k <td></td> <td>10.2 30.0000</td> <td>Calculate plate moment from bolt tension .</td> <td></td>		10.2 30.0000	Calculate plate moment from bolt tension .	
Eccentricity				
A1: Plate Area				
A2 : Support Area	· ·	2 2 M2 0 C C	Stress Ratio	0.000
Support Name	e de late desar a representation	3 18 8 A B B B B B B B B B B B B B B B B B	D	0.000 :-
Calculate plate moment from bearing max(m, n)	A2 : Support Area	144.000 in^2		
Calculate plate moment from bearing	sart(A2/A1)	1.333		C 107 A A 1000
max(m, n) 3.700 in 'A': Bearing Length Bearing Stresses 2.040 ksi Mpl: Plate Moment 0.041 kin Fp: Allowable			Plate Moment from Bolt Tension	0.308 K-In
"A" : Bearing Length Mpl : Plate Moment D. 0.41 k-in D. 0.42 k-in D. 0.490 k-in D. 0.430 D.	Calculate plate moment from bearing			
Mp Plate Moment 0.041 k-in 1.000 1.	* * *	necessary to a		
Stress Ratio	ž ž	receivers som		
Plate Bending Stresses Mmax	MpI : Plate Moment	0.041 k-in		A CONTRACTOR OF THE PROPERTY O
Mmax			Stress Ratio	1.000
Mmax 0.490 k-in ft : Actual 13.946 ksi 5t : Actual 23.400 ksi Stress Ratio 0.430			Plate Bending Stresses	
Fb : Allowable 32.400 ksi				0.490 k-in
Load Comb.: +1.20D+0.50Lr+0.50L+W+1.60H			fb : Actual	13.946 ksi
Load Comb.: +1.20D+0.50Lr+0.50L+W+1.60H Axial Load + Moment, Ecc. > L/2 Loading Pu: Axial			Fb : Allowable	32.400 ksi
Loading Calculate plate moment from bolt tension				
Pu : Axial 0.000 k Tension per Bolt 1.214 k Mu : Moment 1.550 k-ft Tension : Allowable			Stress Ratio	0.430
Pu : Axial 0.000 k Tension per Bolt 1.214 k Mu : Moment 1.550 k-ft Tension : Allowable	Load Comb. : +1.20D+0.50Lr+0	0.50L+W+1.60H		
Mu : Moment 1.550 k-ft Tension : Allowable		0.50L+W+1.60H		Axial Load + Moment, Ecc. > L/2
Eccentricity	Loading		Calculate plate moment from bolt tension .	Axial Load + Moment, Ecc. > L/2
A1 : Plate Area	Loading Pu : Axial	0.000 k	Calculate plate moment from bolt tension . Tension per Bolt	Axial Load + Moment, Ecc. > L/2
A2 : Support Area	Loading Pu : Axial Mu : Moment	0.000 k 1.550 k-ft	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable	Axial Load + Moment, Ecc. > L/2 1.214 k 0.000 k
Sqrt(A2/A1) 1.333 Effective Bolt Width for Bending 9.000 in Plate Moment from Bolt Tension 0.620 k-in	Loading Pu : Axial Mu : Moment Eccentricity	0.000 k 1.550 k-ft 186000.000 in	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable	Axial Load + Moment, Ecc. > L/2 1.214 k 0.000 k
Calculate plate moment from bearing max(m, n) 3.700 in Bearing Stresses "A" : Bearing Length 0.264 in Fp : Allowable 2.040 ksi Mpl : Plate Moment 0.081 k-in fu : Max. Bearing Pressure (set equal to Fp) Stress Ratio 1.000 Plate Bending Stresses Mmax 0.974 k-in	Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable Stress Ratio	Axial Load + Moment, Ecc. > L/2 1.214 k 0.000 k 0.000
max(m, n) 3.700 in Bearing Stresses "A" : Bearing Length 0.264 in Fp : Allowable 2.040 ksi Mpl : Plate Moment 0.081 k-in fu : Max. Bearing Pressure (set equal to Fp) Stress Ratio 1.000 Plate Bending Stresses Mmax 0.974 k-in	Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge	Axial Load + Moment, Ecc. > L/2 1.214 k 0.000 k 0.000 2.300 in
max(m, n) 3.700 in Bearing Stresses "A" : Bearing Length 0.264 in Fp : Allowable 2.040 ksi Mpl : Plate Moment 0.081 k-in fu : Max. Bearing Pressure (set equal to Fp) Stress Ratio 1.000 Plate Bending Stresses Mmax 0.974 k-in	Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2	Calculate plate moment from bolt tension. Tension per Bolt Tension: Allowable Stress Ratio Dist. from Bolt to Col. Edge	Axial Load + Moment, Ecc. > L/2 1.214 k 0.000 k 0.000 2.300 in 9.000 in
"A" : Bearing Length 0.264 in Fp : Allowable	Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1)	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2	Calculate plate moment from bolt tension. Tension per Bolt Tension: Allowable Stress Ratio Dist. from Bolt to Col. Edge	Axial Load + Moment, Ecc. > L/2 1.214 k 0.000 k 0.000 2.300 in 9.000 in
Mpl : Plate Moment 0.081 k-in fu : Max. Bearing Pressure (set equal to Fp) Stress Ratio 1.000 Plate Bending Stresses Mmax 0.974 k-in	Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1) Calculate plate moment from bearing	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge Effective Bolt Width for Bending Plate Moment from Bolt Tension	Axial Load + Moment, Ecc. > L/2 1.214 k 0.000 k 0.000 2.300 in 9.000 in
Stress Ratio	Loading Pu: Axial Mu: Moment Eccentricity A1: Plate Area A2: Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n)	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge Effective Bolt Width for Bending Plate Moment from Bolt Tension	Axial Load + Moment, Ecc. > L/2 1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in
Mmax	Loading Pu: Axial Mu: Moment Eccentricity A1: Plate Area A2: Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n) "A": Bearing Length	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333 3.700 in 0.264 in	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge Effective Bolt Width for Bending Plate Moment from Bolt Tension Bearing Stresses Fp : Allowable	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in
Mmax	Loading Pu: Axial Mu: Moment Eccentricity A1: Plate Area A2: Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n) "A": Bearing Length	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333 3.700 in 0.264 in	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge Effective Bolt Width for Bending Plate Moment from Bolt Tension Bearing Stresses Fp : Allowable fu : Max. Bearing Pressure	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in 2.040 ksi (set equal to Fp)
WITHOUT THE PARTY OF THE PARTY	Loading Pu: Axial Mu: Moment Eccentricity A1: Plate Area A2: Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n) "A": Bearing Length	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333 3.700 in 0.264 in	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge Effective Bolt Width for Bending Plate Moment from Bolt Tension Bearing Stresses Fp : Allowable fu : Max. Bearing Pressure Stress Ratio	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in 2.040 ksi (set equal to Fp)
	Loading Pu: Axial Mu: Moment Eccentricity A1: Plate Area A2: Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n) "A": Bearing Length	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333 3.700 in 0.264 in	Calculate plate moment from bolt tension . Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge Effective Bolt Width for Bending Plate Moment from Bolt Tension Bearing Stresses Fp : Allowable fu : Max. Bearing Pressure Stress Ratio Plate Bending Stresses	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in 2.040 ksi (set equal to Fp) 1.000

Fb: Allowable

Stress Ratio

32.400 ksi 0.855

Project Title: Engineer:CAMPUS BUILDING PERMIT NUMBER B19-163 Project ID: Project Descr:

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Load Comb. : +1.20D+0.50Lr+0).50L-W+1.60H		xial Load + Moment, Ecc. > L/2
Loading	0.000 1-	Calculate plate moment from bolt tension Tension per Bolt	<u>.</u> 1.214 k
Pu : Axial	0.000 k	Tension : Allowable	
Mu : Moment Eccentricity	1.550 k-ft 186000.000 in	Stress Ratio	0.000 k 0.000
	81.000 in^2	ouess Nado	0.000
A1 : Plate Area A2 : Support Area	144.000 in^2	Dist. from Bolt to Col. Edge	2.300 in
The country of Transaction of the Country of the Co		Effective Bolt Width for Bending	9.000 in
sqrt(A2/A1)	1.333	Plate Moment from Bolt Tension	0.620 k-in
Calculate plate moment from bearing			
max(m, n)	3.700 in	Bearing Stresses	
"A" : Bearing Length	0.264 in	Fp : Allowable	2.040 ksi
Mpl : Plate Moment	0.081 k-in	fu : Max. Bearing Pressure	(set equal to Fp)
		Stress Ratio	1.000
		Plate Bending Stresses	
		Mmax	0.974 k-in
		fb : Actual	27.712 ksi
		Fb : Allowable	32.400 ksi
		Stress Ratio	0.855
			vial Lood + Moment Foe > 1/2
Load Comb. : +1.20D+0.50L+0.	50S+W+1.60H	<i>F</i>	xial Load + Moment, Ecc. > L/2
	50S+W+1.60H		
Loading	50S+W+1.60H 0.000 k	Calculate plate moment from bolt tension Tension per Bolt	
		Calculate plate moment from bolt tension	<u>.</u>
Loading Pu : Axial	0.000 k	Calculate plate moment from bolt tension Tension per Bolt	<u>.</u> 1.214 k
<u>Loading</u> Pu : Axial Mu : Moment	0.000 k 1.550 k-ft	Calculate plate moment from bolt tension Tension per Bolt Tension : Allowable Stress Ratio	1.214 k 0.000 k 0.000
<u>Loading</u> Pu : Axial Mu : Moment Eccentricity	0.000 k 1.550 k-ft 186000.000 in	Calculate plate moment from bolt tension Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge	1.214 k 0.000 k 0.000 2.300 in
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2	Calculate plate moment from bolt tension Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge Effective Bolt Width for Bending	1.214 k 0.000 k 0.000 2.300 in 9.000 in
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1)	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2	Calculate plate moment from bolt tension Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge	1.214 k 0.000 k 0.000 2.300 in
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1) Calculate plate moment from bearing	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2	Calculate plate moment from bolt tension Tension per Bolt	1.214 k 0.000 k 0.000 2.300 in 9.000 in
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n)	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333	Calculate plate moment from bolt tension Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge Effective Bolt Width for Bending Plate Moment from Bolt Tension Bearing Stresses	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n) "A" : Bearing Length	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333	Calculate plate moment from bolt tension Tension per Bolt Tension : Allowable Stress Ratio Dist. from Bolt to Col. Edge Effective Bolt Width for Bending Plate Moment from Bolt Tension Bearing Stresses Fp : Allowable	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n)	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333	Calculate plate moment from bolt tension Tension per Bolt	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in 2.040 ksi (set equal to Fp)
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n) "A" : Bearing Length	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333	Calculate plate moment from bolt tension Tension per Bolt	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n) "A" : Bearing Length	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333	Calculate plate moment from bolt tension Tension per Bolt	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in 2.040 ksi (set equal to Fp) 1.000
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n) "A" : Bearing Length	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333	Calculate plate moment from bolt tension Tension per Bolt	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in 2.040 ksi (set equal to Fp) 1.000 0.974 k-in
Loading Pu : Axial Mu : Moment Eccentricity A1 : Plate Area A2 : Support Area sqrt(A2/A1) Calculate plate moment from bearing max(m, n) "A" : Bearing Length	0.000 k 1.550 k-ft 186000.000 in 81.000 in^2 144.000 in^2 1.333	Calculate plate moment from bolt tension Tension per Bolt	1.214 k 0.000 k 0.000 2.300 in 9.000 in 0.620 k-in 2.040 ksi (set equal to Fp) 1.000

Fb : Allowable Stress Ratio

0.855

Project Title: EngineerCAMPUS BUILDING PERMIT NUMBER B19-163 Project ID: Project Descr:

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Load Comb. : +1.20D+0.50L+0.	.50S-W+1.60H		Axial Load + Moment, Ecc. > L/2
Loading		Calculate plate moment from bolt tension .	
Pu : Axial	0.000 k	Tension per Bolt	1.214 k
Mu : Moment	1.550 k-ft	Tension : Allowable	0.000 k
Eccentricity	186000.000 in	Stress Ratio	0.000
A1 : Plate Area	81.000 in^2		
A2 : Support Area	144.000 in^2	Dist. from Bolt to Col. Edge	2.300 in
	4 222	Effective Bolt Width for Bending	9.000 in
sqrt(A2/A1)	1.333	Plate Moment from Bolt Tension	0.620 k-in
Calculate plate mamont from bearing			
Calculate plate moment from bearing	0.700 /	D 1 0	
max(m, n)	3.700 in	Bearing Stresses	
"A" : Bearing Length	0.264 in	Fp : Allowable	2.040 ksi
Mpl : Plate Moment	0.081 k-in	fu : Max. Bearing Pressure	(set equal to Fp)
		Stress Ratio	1.000
		Plate Bending Stresses	
		Mmax	0.974 k-in
		fb : Actual	27.712 ksi
		Fb : Allowable	32.400 ksi
		Stress Ratio	0.855
1 10 1 14 00 5 10 501 10	000.5.4.001		
Load Comb. : +1.20D+0.50L+0.	20S+E+1.60H		Axial Load Only, No Moment
<u>Loading</u>		Bearing Stresses	
Pu : Axial	0.000 k	Fp : Allowable	2.040 ksi
Design Plate Height	9.000 in	fu : Max. Bearing Pressure	0.000 ksi
Design Plate Width	9.000 in	Stress Ratio	0.000
Will be different from entry if partial bearing used.	0.000 [[]		0.000
, , , ,		Plate Bending Stresses	
A1 : Plate Area	81.000 in^2	Mmax = Fu * L^2 / 2	0.000 k-in
A2: Support Area	144.000 in^2	fb : Actual	0.000 ksi
sqrt(A2/A1)	1.333	Fb : Allowable	32.400 ksi
sqrt(AZIAT)	1.555	Stress Ratio	0.000
Distance for Memont Calculation		Oli Oli Pialio Illinininini	01000
Distance for Moment Calculation	2 550 :-		
" m "	3.550 in		
" n "	3.550 in		
X	0.000 in^2		
Lambda	0.000		
n'	0.000 in		
n' * Lambda	0.000 in		
L = max(m, n, n")	3.550 in		
Load Comb. : +0.90D+W+0.90H	ł		Axial Load + Moment, Ecc. > L/2
Loading		Calculate plate moment from bolt tension .	
Pu : Axial	0.000 k	Tension per Bolt	1.214 k
Mu : Moment	1.550 k-ft	Tension : Allowable	0.000 k
Eccentricity	186000.000 in	Stress Ratio	0.000 k 0.000
		Stress Ratio	0.000
A1 : Plate Area	81.000 in^2		5 444 to
A2 : Support Area	144.000 in^2	Dist. from Bolt to Col. Edge	2.300 in
and (AO/A1)	4 222	Effective Bolt Width for Bending	9.000 in
sqrt(A2/A1)	1.333	Plate Moment from Bolt Tension	0.620 k-in
Calculate plate moment from bearing			
	2 700 in	Dearing Ctrosses	
max(m, n)	3.700 in	Bearing Stresses	0.040 !: !
"A" : Bearing Length	0.264 in	Fp : Allowable	2.040 ksi
Mpl : Plate Moment	0.081 k-in	fu : Max. Bearing Pressure	(set equal to Fp)
520		Stress Ratio	1.000
		Plate Bending Stresses	
		Mmax	0.974 k-in
		fb : Actual	27.712 ksi
		Fb : Allowable	32.400 ksi
		Stress Ratio	0.855
		Out 033 Natio	0.000

Project Title: EngineerCAMPUS BUILDING PERMIT NUMBER B19-163
Project ID: Project Descr:

Printed: 7 MAR 2019, 3:24PM

Steel Base Plate

Lic. #: KW-06000933

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Load Comb.: +0.90D-W	+0	1.90H
----------------------	----	-------

Loading		
Pu : Axial	0.000	k
Mu : Moment	1.550	k-ft
Eccentricity	186000.000	in
A1 : Plate Area	81.000	in^2
A2 : Support Area	144.000	in^2
sqrt(A2/A1)	1.333	
Calculate plate moment from bearing	3.700	in

C

alculate plate moment from bearing	
max(m, n)	3.700 in
"A" : Bearing Length	0.264 in
Mpl : Plate Moment	0.081 k-in

Load Comb.: +0.90D+E+0.90H

Loading Pu: Axial Design Plate Height Design Plate Width Will be different from entry if partial bearing used.	0.000 k 9.000 in 9.000 in
A1 : Plate Area	81.000 in^2
A2: Support Area	144.000 in^2
sqrt(A2/A1)	1.333
Distance for Moment Calculation	
" m "	3.550 in
" n "	3.550 in
X	0.000 in^2
Lambda	0.000
n'	0.000 in
n' * Lambda	0.000 in
L = max(m, n, n")	3.550 in

Axial Load + Moment, Ecc. > L/2

Calculate plate moment from bolt tension Tension per Bolt Tension : Allowable Stress Ratio	1.214 k 0.000 k 0.000
Dist. from Bolt to Col. Edge Effective Bolt Width for Bending Plate Moment from Bolt Tension	2.300 in 9.000 in 0.620 k-in
Bearing Stresses Fp : Allowable fu : Max. Bearing Pressure Stress Ratio	2.040 ksi (set equal to Fp) 1.000
Plate Bending Stresses Mmax fb : Actual Fb : Allowable Stress Ratio	0.974 k-in 27.712 ksi 32.400 ksi 0.855

Axial Load Only, No Moment

Bearing Stresses	
Fp : Allowable	2.040 ksi
fu : Max. Bearing Pressure	0.000 ksi
Stress Ratio	0.000
Plate Bending Stresses	
Mmax = Fu * L^2 / 2	0.000 k-ir
fb : Actual	0.000 ksi
Fb : Allowable	32.400 ksi
Stress Ratio	0.000

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1.Project information

Customer company: Customer contact name: Customer e-mail: Comment:

Location: Fastening description:

Project description:

2. Input Data & Anchor Parameters

General

Design method:ACI 318-14 Units: Imperial units

Anchor Information:

Anchor type: Torque controlled expansion anchor

Material: Carbon Steel Diameter (inch): 0.375

Nominal Embedment depth (inch): 2.750 Effective Embedment depth, hef (inch): 2.375

Code report: ICC-ES ESR-3037

Anchor category: 1 Anchor ductility: Yes h_{min} (inch): 4.34 cac (inch): 6.06 Cmin (inch): 6.00 S_{min} (inch): 3.00

Base Material

Concrete: All-lightweight Concrete thickness, h (inch): 8.00 State: Cracked

Compressive strength, f'c (psi): 3000

Ψ_{c,V}: 1.0

Reinforcement condition: B tension, B shear Supplemental reinforcement: Not applicable Reinforcement provided at corners: No Ignore concrete breakout in tension: No Ignore concrete breakout in shear: No Ignore 6do requirement: Not applicable

Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 9.00 x 0.38

Recommended Anchor

Anchor Name: Strong-Bolt® 2 - 3/8"Ø CS Strong-Bolt 2, hnom:2.75" (70mm)

Code Report: ICC-ES ESR-3037



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1.Project information

Customer company: Customer contact name: Customer e-mail: Comment: Project description: Location: Fastening description:

2. Input Data & Anchor Parameters

General

Design method:ACI 318-14 Units: Imperial units

Anchor Information:

Anchor type: Torque controlled expansion anchor

Material: Carbon Steel Diameter (inch): 0.375

Nominal Embedment depth (inch): 2.875 Effective Embedment depth, her (inch): 2.500

Code report: ICC-ES ESR-3037

Anchor category: 1 Anchor ductility: Yes h_{min} (inch): 4.50 c_{ac} (inch): 6.00 C_{min} (inch): 6.00 S_{min} (inch): 3.00

Base Material

Concrete: All-lightweight

Concrete thickness, h (inch): 8.00

State: Cracked

Compressive strength, f'c (psi): 3000

Ψ_{c,V}: 1.0

Reinforcement condition: B tension, B shear Supplemental reinforcement: Not applicable Reinforcement provided at corners: No Ignore concrete breakout in tension: No Ignore concrete breakout in shear: No Ignore 6do requirement: Not applicable

Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): $9.00 \times 9.00 \times 0.50$

Recommended Anchor

Anchor Name: Strong-Bolt® 2 - 3/8"Ø CS Strong-Bolt 2, hnom:2.875" (73mm)

Code Report: ICC-ES ESR-3037





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Load and Geometry

Load factor source: ACI 318 Section 5.3

Load combination: not set Seismic design: Yes

Anchors subjected to sustained tension: Not applicable Ductility section for tension: 17.2.3.4.3 (c) is satisfied Ductility section for shear: 17.2.3.5.3 (b) is satisfied

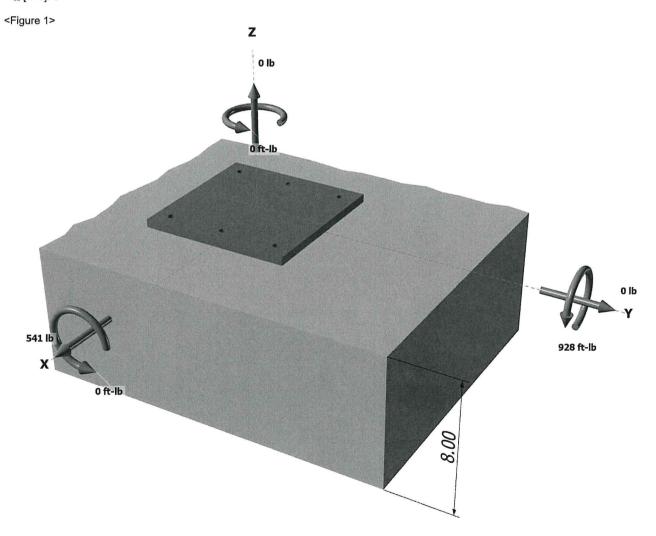
Ω₀ factor: not set

Apply entire shear load at front row: No

Anchors only resisting wind and/or seismic loads: Yes

Strength level loads:

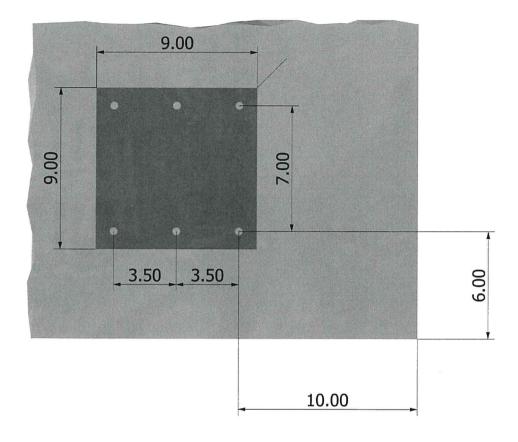
Nua [lb]: 0 Vuax [lb]: 541 Vuay [lb]: 0 Mux [ft-lb]: 0 Muy [ft-lb]: 928 Muz [ft-lb]: 0





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<Figure 2>





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2+(V_{uay})^2}$ (Ib)	
1	493.3	90.2	0.0	90.2	
2	493.3	90.2	0.0	90.2	
3	493.3	90.2	0.0	90.2	
4	0.0	90.2	0.0	90.2	
5	0.0	90.2	0.0	90.2	
6	0.0	90.2	0.0	90.2	
Sum	1480.0	541.0	0.0	541.0	

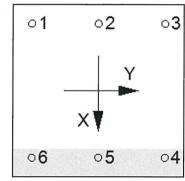
Maximum concrete compression strain (%): 0.05

Maximum concrete compression stress (psi): 228

Resultant tension force (lb): 1480 Resultant compression force (lb): 1480

Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. 17.4.1)

N _{sa} (lb)	ϕ	ϕN_{sa} (lb)	
5600	0.75	4200	

5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.4.2)

 $N_b = k_c \lambda_a \sqrt{f'_c h_{ef}}^{1.5}$ (Eq. 17.4.2.2a)

Kc	λa	Tc (psi)	n _{ef} (in)	Νь ((מו					
17.0	0.60	3000	2.500	220	8					
$0.75\phi N_{cbg} =$	=0.75¢ (Anc/An	Vco) Ψ ec,N Ψ ed,N Ψ c	$_{c,N} arPsi_{cp,N} N_b$ (Sec	. 17.3.1 & E	q. 17.4.2.1b)					
A _{Nc} (in ²)	A _{Nco} (in²)	C _{a,min} (in)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	$0.75\phi N_{cbg}$ (lb)	
108.75	56.25	10.00	1.000	1.000	1.00	1.000	2208	0.65	2081	_

A / /II- \

6. Pullout Strength of Anchor in Tension (Sec. 17.4.3)

 $0.75\phi N_{pn} = 0.75\phi \Psi_{c,P} \lambda_a N_p (f'_c/2,500)^n$ (Sec. 17.3.1, Eq. 17.4.3.1 & Code Report)

$\Psi_{c,P}$	λa	N_P (lb)	f_c (psi)	n	ϕ	$0.75\phi N_{pn}$ (lb)
1.0	0.60	2775	3000	0.50	0.65	889



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8. Steel Strength of Anchor in Shear (Sec. 17.5.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (Ib)	
1800	1.0	0.65	1170	

9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.5.2)

Shear perpendicular to edge in x-direction:

 $V_{bx} = \min |7(I_e/d_a)^{0.2} \sqrt{d_a \lambda_a} \sqrt{f'_c c_{a1}}^{1.5}; \ 9\lambda_a \sqrt{f'_c c_{a1}}^{1.5}| \ (\text{Eq. 17.5.2.2a \& Eq. 17.5.2.2b})$

l _e (in)	da (in)	λ_a	f'c (psi)	Ca1 (in)	V_{bx} (lb)			
2.50	0.375	0.60	3000	13.00	9650			
$\phi V_{cbgx} = \phi (A_1)$	vc / Avco) Yec, v Ye	$_{ed,V} \mathscr{V}_{c,V} \mathscr{V}_{h,V} V_{bx}$	(Sec. 17.3.1 & E	q. 17.5.2.1b)				
A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
292.00	760.50	1.000	0.854	1.000	1 561	9650	0.70	3457

Shear parallel to edge in x-direction:

 $V_{by} = \min |7(I_e/d_a)^{0.2} \sqrt{d_a \lambda_a} \sqrt{f'_c c_{a1}}^{1.5}; \ 9\lambda_a \sqrt{f'_c c_{a1}}^{1.5}| \ (\text{Eq. 17.5.2.2a \& Eq. 17.5.2.2b})$

76 (III)	Ga (III)	Λa	7 c (psi)	Car (III)	V by (ID)			
2.50	0.375	0.60	3000	10.00	6510			
$\phi V_{cbgx} = \phi (2$	2)(Avc / Avco) Yec,1	$VY_{ed,V}Y_{c,V}Y_{h,V}$	/ _{by} (Sec. 17.3.1,	17.5.2.1(c) & E	q. 17.5.2.1b)			
Av_c (in ²)	A_{Vco} (in ²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
224.00	450.00	1.000	1.000	1.000	1.369	6510	0.70	6213

10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.5.3)

 $\phi V_{cpg} = \phi k_{cp} N_{cbg} = \phi k_{cp} (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b$ (Sec. 17.3.1 & Eq. 17.5.3.1b)

Kcp	A_{Nc} (in ²)	A _{Nco} (in ²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{c ho,N}$	N_b (lb)	ϕ	ϕV_{cpg} (lb)
2.0	210.25	56.25	1.000	1.000	1.000	1.000	2208	0.70	11556

11. Results

Interaction of Tensile and Shear Forces (Sec. 17.6.)

Tension	Factored Lo	ad, N _{ua} (lb)	Design St	rength, øN₁ (lb)	Ratio		Status
Steel	493		4200		0.12		Pass
Concrete breakout	1480		2081		0.71		Pass (Governs)
Pullout	493		889		0.55		Pass
Shear	Factored Lo	ad, V _{ua} (lb)	Design St	rength, øVn (lb)	Ratio	6	Status
Steel	90		1170		0.08		Pass
T Concrete breakout	x+ 541		3457		0.16		Pass (Governs)
Concrete breakout	y+ 180		6213		0.03		Pass (Governs)
Pryout	541		11556		0.05		Pass
Interaction check	Nua/ØNn	Vua/øVn		Combined Ratio)	Permissible	Status
Sec. 17.61	0.71	0.00		71.1%		1.0	Pass

3/8"Ø CS Strong-Bolt 2, hnom:2.875" (73mm) meets the selected design criteria.



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12. Warnings

- Minimum spacing and edge distance requirement of 6da per ACI 318 Sections 17.7.1 and 17.7.2 for torqued cast-in-place anchor is waived per designer option.
- Per designer input, ductility requirements for tension have been determined to be satisfied designer to verify.
- Per designer input, ductility requirements for shear have been determined to be satisfied designer to verify.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.